

# TOWN OF SPRINGDALE CULINARY WATER MASTER PLAN

*Including:*

*Five Point Analysis*

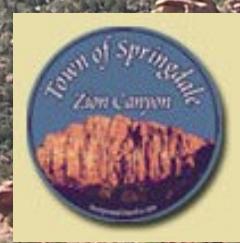
*Recommended System Improvements  
and*

*Written Analysis for Water User Rate & Impact Fee*

**PREPARED BY:**



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**July 2008**

# SPRINGDALE, UTAH CULINARY WATER MASTER PLAN

**July 2008**

**INCLUDES:**

**FIVE POINT ANALYSIS  
RECOMMENDED SYSTEM IMPROVEMENTS  
AND**

**WRITTEN ANALYSIS FOR WATER USER RATE AND IMPACT FEE**

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COUNCIL MEMBER-----	Louise Excell
COUNCIL MEMBER-----	Kathy LaFave
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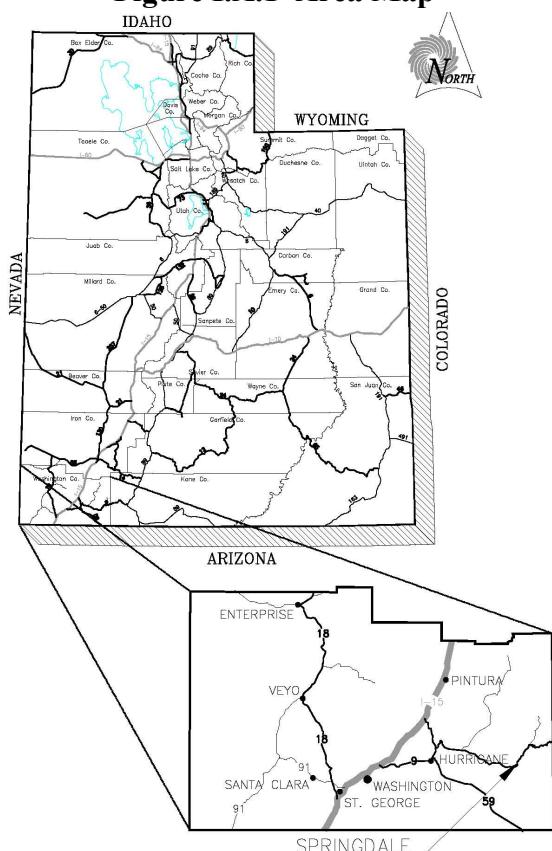
## **SECTION I INTRODUCTION**

## A. PREFACE

*On April 9, 2008 the Town of Springdale contracted with Sunrise Engineering, Inc. to provide a Culinary Water Master Plan that would address needs of the culinary water system for 20 year planning period and also for anticipated Town buildout. Needed water rights and storage for anticipated buildout are of particular interest to the Town at this time, so that any improvements made today will have the ability to service the Town to the anticipated buildout.*

*A glance at the Springdale Culinary Water System shows a number of different sources and associated water rights. Historically these different sources have had times of use and times of no use depending on need and*

**Figure I.A.1 Area Map**



*feasibility of use. Currently the Town uses water diverted from the Virgin River to meet both culinary and secondary needs. Additional water can be obtained from other sources during periods of high use. Culinary water is treated by a filtration treatment plant before being introduced into the culinary system.*

The existing storage and distribution system layout has been in place for some time and is in need of some changes that could be made to bring the service up to current capacity state standards. Many needed improvements have been recognized by the Town and verified by this report. Those recognized improvements include a new tank located at a higher elevation, preferably at the elevation of the existing Anasazi tank, a new line from the proposed tank to the existing system, and replacement of transite and undersized lines.

## B. INTRODUCTION

This Culinary Water Master Plan has been prepared for the Town of Springdale, located in Washington County Utah, East of St. George, Utah along Highway 9 and adjacent to Zion National Park. The Town of Springdale has experienced moderate to high growth rates for a small town over the past 20 years. As in other communities, the culinary water system must be improved and enlarged to support growth and development within the Town, and to comply with current State of Utah Drinking water Standards. Unlike many small towns of similar size, Springdale has experienced much of its recent growth in commercial use such as hotels and restaurants. This growth along with the associated residential growth experienced presents a challenge for the planning of infrastructure to accommodate existing and future growth.

The culinary water system has been analyzed under The State of Utah Department of Drinking Water Regulations to determine existing system conditions and needs, and to

determine projected system needs as the community grows during the next 20 years and to an anticipated buildout. The culinary water system improvements have been recommended, and a proposed financing plan has been developed.

Springdale average water rates and impact fees have also been analyzed in support of the proposed financing plan and the recommended system improvements. The recommended culinary water rates and impact fees are fair, and they will allow the Town to continue to maintain the level of service that is required of public water systems for the present and over the duration of the 20 year planning period and beyond.

## SECTION II

### SYSTEM USERS ANALYSIS

#### A. PROJECTED GROWTH RATE

An essential element in the development of a Culinary Water Master Plan is the projection of the Town's growth rate to an anticipated buildout if possible. The population growth rate gives the planner a glimpse of the future demands that may need to be accommodated by the Town's culinary water system.

Projecting the number of future culinary water connections with any degree of accuracy can be a very subjective process, especially with the variety of connections serviced by the Town of Springdale. Springdale currently services connections ranging from seasonal residential to large 120+ room hotels with high usage during tourist season. With this variation in mind this plan uses population and zoning data in line with the Town's General Plan to estimate growth in the 20 year planning period. Table II.A.1 shows the historic growth rate and provides an idea of how the community has grown based on Census counts from 1970 through 2000, and Census estimates from 2006.

**Table II.A.1 Sprindale Historic Population**

Year	US Census Population	Annual Growth Rate	
1970	182	-	-
1980	258	1970-1980	3.6%
1990	275	1980-1990	0.6%
2000	457	1990-2000	5.2%
2006 (Est.)	550	2000-2006	3.1%

While population growth is essential to anticipating water needs over a specific time period, eventually development in the Town will reach the boundaries currently defined by Zion National Park on the North, East and West. The estimated buildout of the Town reflects the condition when development has reached these extents defined by Zion National

Park. Town staff has prepared anticipated buildout numbers which will be used as part of this Master Plan. Table II.A.2 shows the anticipated ERUs buildout based upon zoning, current ERU data and known developments in

**Table II.A.2 Estimated Buildout**

Residential ERUs	1,022	ERUs
Commercial ERUs	998	ERUs
Other ERUs	100	ERUs
Total	2,120	ERUs

the Town. While the exact number of ERUs seen by the Town may not exactly match those shown in Table II.A.2 the numbers do allow the Town to examine the anticipated maximum improvements that would be needed to service the Town to the extent that no further growth is anticipated.

It is important to understand that projected population figures tied to the growth rate shown are not the corner stone of this master plan. The Town of Springdale is expected to experience commercial growth that could vary greatly from the growth anticipated with a simple growth rate. Commercial growth is also expected to have a greater overall impact on the demands of the water system over the planning period. Recommended improvements are based upon buildout ERUs which may be achieved sooner or later than anticipated. If the number of ERUs projected changes on a small scale, then future improvements to support growth may also be affected on a small scale and could be addressed in updates to this master plan. Large scale changes such as annexation of large parcels outside the existing Town boundaries are not expected, but could be addressed in a future updated master plan. Impact Fees should not be significantly affected if buildout conditions are reached sooner or later than anticipated.

#### B. LENGTH OF PLANNING PERIOD

This culinary water master plan uses two planning periods for analysis and recommended

improvements. A 20-year planning period, beginning in fiscal year 2008, July 1 of 2007, and running through fiscal year 2028 is used to show growth similar to that experienced in the past by the Town. A buildout planning period is also included to show anticipated connections and improvements needed to meet water system demands up to the time when Springdale has been developed to the current Town boundaries. While the period of time to the buildout condition is not currently known, it is expected to be beyond the initial 20 year planning period. Revenue sources should be carefully evaluated each year as the Town Council sets budgets and anticipates system requirements.

## C. CULINARY WATER CONNECTIONS

### 1. Existing Culinary Water Connections

The State of Utah Administrative Rules for Public Drinking Water Systems requires public water systems to meet requirements based upon usage. State rules provide a standard usage based upon the types of connections serviced in a system. Usage can also be based upon historical data if there is enough data to provide a confidence level of 90% or higher that the usage shown is representative of actual average use.

The Town of Springdale has provided historic usage which will meet the confidence requirements outlined previously. The analysis of this historic usage will be outlined in this section.

According to the Town of Springdale Rate Table Summary data, the average number of existing culinary connections in FY 2007 was 322. The 322 connections include 222 residential connections, 95 commercial connections and 5 other connections.

To calculate how much water is used at an average residential connection, the total amount of water used by all Springdale residential customers over the course of a year was determined. Table II.C.1 below provides historic data from Springdale records from 2005 to 2008. The average daily use per residential connection was **156 gal/day**.

In comparison to other communities of similar nature, the daily average use for Springdale appears to be significantly lower. This is likely due to the many seasonal homes, use of xeriscape landscaping, and restrictions placed on outdoor use of the culinary water system. Also, the Springdale Consolidated Irrigation Company and the Town both serve irrigation connections for secondary use throughout the Town. With this uniquely low outdoor usage in mind it is recommended that any future

**TABLE II.C.1 Average Usage Per Connection**

	2005	2006	2007	2008 *	Average
<b>Residential</b>					<b>05 - 07</b>
Usage (sum of metered use) (gallons)	13,186,000	11,552,000	12,209,000	2,131,000	12,315,667
Connections (ERUs)	210	217	222	225	216
Usage Per Connection (gal/year)	62,790	53,235	54,995	9,471	57,007
Daily Usage Per Connection (gal/day)	172	146	151	158	<b>156</b>
This master plan will use a historical daily ERU usage of					250 gpd/conn.

\* Year 2008 is a partial year usage and was not used in the analysis

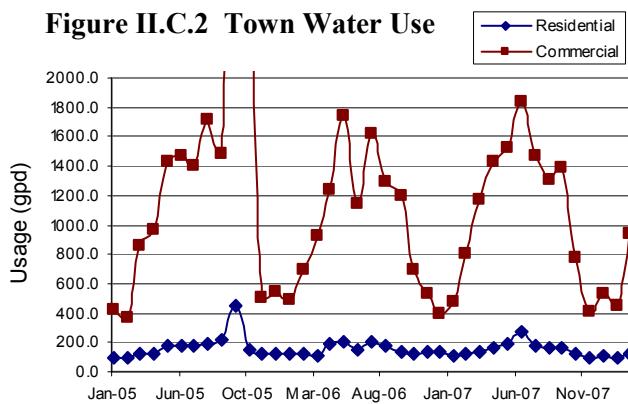
improvements be sized for a conservative 250 gal/day per ERU. The remainder of this report will refer to this usage as historical usage and each section will include analysis showing requirements based upon this historic usage.

Commercial connections generally require more water than that required by a residential customer. An Equivalent Residential Unit (ERU) represents the additional volume of water required for commercial users above and beyond the amount used by an average residential connection. The ERU value is determined by comparing the average daily use per commercial connection to the average daily use per residential connection. To calculate the average daily use for commercial connections, the total amount of water used by all commercial users was determined for the usage data period. In the years 2005-2007 the average commercial water usage was approximately 40,727,000 gallons, distributed to an *average of 94 commercial users*. The average usage per commercial connection between 2005 and 2007 was 1,187 gal/day. Below is the calculation showing average commercial usage.

Using the adjusted historic usage of 250 gpd/ connection for residential use, the average commercial connection uses approximately 4.74 times the amount used by the average residential connection. Thus, for the purpose of this master plan an ERU value for each historic usage commercial connection will be 4.74 times that of a residential connection.

Similar analysis of the connections designated as “other” reveals average “other” usage of only 190,333 gallons from 2005-2007 with an average of 4 connections. Other connections have an average daily use of 130 gpd during this period. Because “other” per connection usage is less than that of a residential connection this master plan will round each

**Figure II.C.2 Town Water Use**



“other” connection to 1 ERU in historic usage.

Shown below is the number of current (March 2008) residential connections, commercial and other ERUs. This represents the method in which the total number of ERUs were calculated.

#### ERUs 2008 (Historic Usage 250 gpd)

Residential	= 229
Commercial ERUs (98 X 4.74)	= 464
Other ERUs (6 X 1.0)	= 6
Total ERUs	= 699

## 2. Projected Culinary Water Connections and ERUs

The number of future culinary connections for each year can be calculated using the compound interest formula and inserting the projected growth rate, the existing number of culinary water ERUs, and the 20-year planning period for culinary water improvements.

$$F = P (1 + i)^N$$

F = Future Population

P = Present Population

i = Historic Growth Rate

N = Years

$$2005 - 2007 \text{ Average Commercial Use} = \frac{40,727,000 \text{ Gallons}}{1 \text{ Year}} \times \frac{1 \text{ Year}}{365 \text{ Days}} \times \frac{1}{94 \text{ Conn.}} = 1,187 \text{ Gal / Day / Conn.}$$

The 20-Year projection of ERUs can be found in Table II.C.3. As shown, the total number of culinary water ERUs projected for the end of the 20-year planning period in 2028 is 1,289 according to historic usage. Note that for this analysis the commercial growth rate is equivalent to the residential growth rate. It is recommended that the Town of Springdale size

all future culinary water related infrastructure improvements for at least 1,289 ERUs.

**Table II.C.3 Residential Water Use (At End of Each Year Shown)**

Year	Est. Residential Growth Rate	Total Estimated Connections	Total Estimated ERUs (Historical Usage)	Estimated Population **
2005	-	307	654	532
2006 *	-	316	671	550
2007	-	322	677	563
2008	3.1%	332	699	580
2009	3.1%	343	720	598
2010	3.1%	353	742	616
2011	3.1%	364	766	636
2012	3.1%	376	790	656
2013	3.1%	388	814	677
2014	3.1%	399	839	697
2015	3.1%	412	866	720
2016	3.1%	425	893	743
2017	3.1%	438	920	765
2018	3.1%	452	949	788
2019	3.1%	466	978	814
2020	3.1%	480	1,009	839
2021	3.1%	495	1,040	864
2022	3.1%	511	1,073	892
2023	3.1%	527	1,106	920
2024	3.1%	543	1,140	948
2025	3.1%	560	1,176	978
2026	3.1%	578	1,212	1,009
2027	3.1%	595	1,249	1,039
2028	3.1%	614	1,289	1,072

Estimated ERUs for 2005-2007 are based upon actual connection data, 2008-2028 ERUs are based upon the growth rate shown for each year and are an end of year ERU estimate

\* 2006 Population data is based on an estimate provided by the US Census.

\*\* Estimated Population is determined by multiplying the Estimated Residential ERUs by a calculated average household size of 2.53. The value 2.53 was obtained by taking the population estimate of 2006 and dividing it by the estimated residential ERUs.

550 people / 217 residential ERUs = 2.53 people per residential ERU.

## SECTION III

### WATER RIGHTS ANALYSIS

#### A. EXISTING WATER RIGHT

The existing Town of Springdale water rights used for culinary water are identified in Table III.A.1 below. The water rights are listed according to number, source, and flow.

#### B. EXISTING REQUIRED WATER RIGHT

The State of Utah Public Drinking Water Regulations, Section 5, states that a community should have adequate water right to supply each culinary connection with 400 gallons per day for indoor water use, plus an amount for outdoor use as dictated by irrigated acreage and a consumptive use value obtained from the State Guidelines. The community may substitute historical use data for indoor and outdoor requirements. For planning purposes the Town of Springdale historic average daily use is assumed to be 250 gallons per ERU per day as outlined in Section II. In the Town of Springdale where secondary irrigation is available and encouraged these amounts are assumed to include all indoor and outdoor usage from the culinary system.

From Table III.A.1, the total amount of water right available in Springdale is 1,182.6 acre-feet. Based on the historic average daily use of 250 gallons per day per ERU and a total of 699 existing ERUs, the existing required water right is calculated as follows:

Existing required water right (FY 2008):  
**Historic Usage**

$$699 \text{ ERUs} \times \frac{250 \text{ gpd}}{\text{ERU}} \times \frac{1 \text{ day}}{1,440 \text{ min.}} = 121 \text{ gpm}$$

$$121 \text{ gpm} \times \frac{1.61 \text{ ac - ft}}{\text{gpm}} = 196 \text{ ac - ft}$$

The existing water right surplus or deficit under these conditions is determined by subtracting the existing required water right of 196 ac-ft from the total available water right of 1,182.6 ac-ft, which yields a surplus of 987 ac-ft.

#### C. PROJECTED REQUIRED WATER RIGHT

The projected required water right at the end of the 20 year planning period is calculated by using the same factors as above, but the projected number of culinary water ERUs are

**Table III.A.1 Town of Springdale Culinary Water Rights**

W.R. #	Source	Flow		
		gpm	cfs	AcFt.
81-105	Spring above ZNP Campground	7.2	0.016	11.58
81-220	Birch Springs East - West of ZNP Museum	18.8	0.042	30.41
81-274	Birch Springs West - West of ZNP Museum	31.4	0.070	50.68
81-585	Hummingbird Well	148.1	0.330	238.91
81-1326	Cemetery Well	65.1	0.145	104.98
81-2413	Big Springs	235.6	0.525	380.08
81-3392	Springdale Town for Municipal Use - Irrigation	596.9	1.330	365.95
<b>Total Water Rights =</b>		<b>1,103.2</b>	<b>2.458</b>	<b>1,182.6</b>

substituted in the calculations. Two different scenarios will be considered in the projection analysis; the 20-year historic usage and buildout historic usage. The historic average daily use of gallons per day per ERU will remain the same at 250. The projected 20 year estimates of ERUs are 1,289 ERUs based on historic and 2,120 ERUs based on the Town's calculated buildout scenario. The calculations for the projected water right requirements are as follows:

Projected required water right (FY 2028):

**Historic Usage**

$$1,289 \text{ ERUs} \times \frac{250 \text{ gpd}}{\text{ERU}} \times \frac{1\text{day}}{1,440 \text{ min.}} = 224 \text{ gpm}$$

$$224 \text{ gpm} \times \frac{1.61 \text{ ac - ft}}{\text{gpm}} = 361 \text{ ac - ft}$$

The projected water right surplus or deficit is determined by subtracting the projected required water right of 361 ac-ft from the grand total available water right of 1,182.6 ac-ft, which yields a surplus of 822 ac-ft.

Projected required water right (Buildout):

**Historic Usage**

$$2,120 \text{ ERUs} \times \frac{250 \text{ gpd}}{\text{ERU}} \times \frac{1\text{day}}{1,440 \text{ min.}} = 368 \text{ gpm}$$

$$368 \text{ gpm} \times \frac{1.61 \text{ ac - ft}}{\text{gpm}} = 594 \text{ ac - ft}$$

The projected water right results in a surplus of 589 ac-ft.

A summarization of all scenarios previously calculated is found at the end of this section.

## D. RECOMMENDED WATER RIGHT IMPROVEMENTS

The projections in this analysis show that the Town of Springdale has enough water rights to

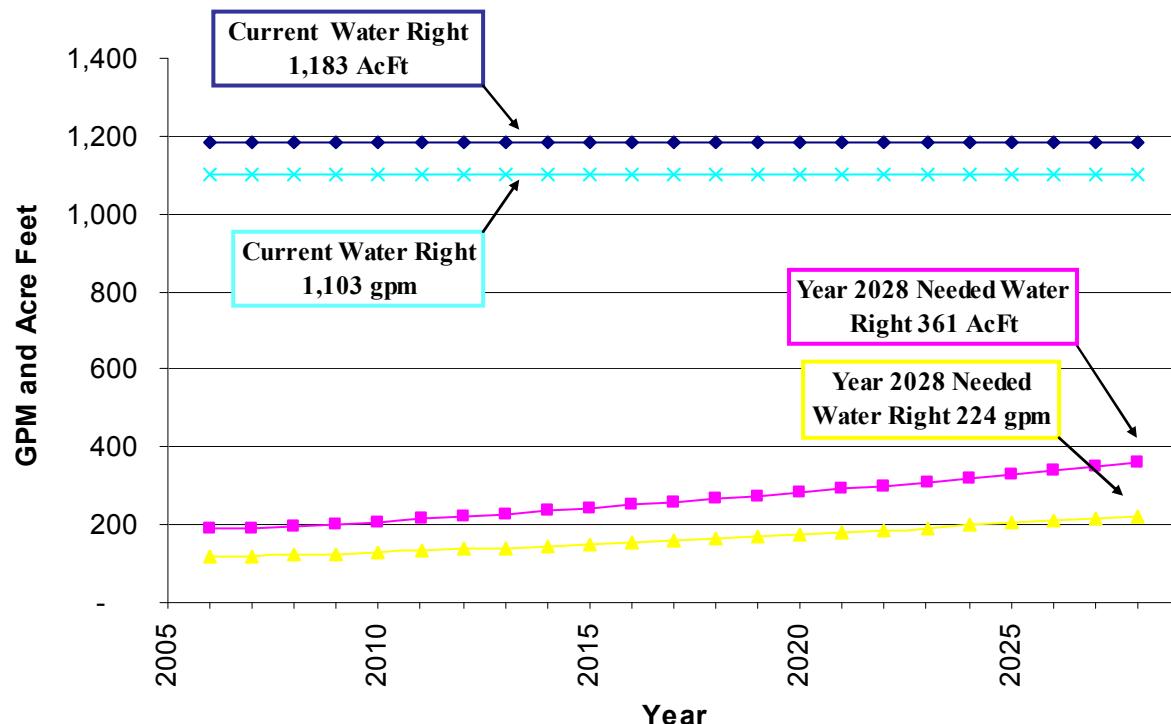
serve the Town including planned development through a 20 year planning period and buildout. However, it is assumed in all previous calculations that at Town buildout all culinary connections will also have an irrigation connection and that the irrigation water rights are sufficient to supply the Towns irrigation needs. It is recommended that an additional study be conducted to insure the Towns Water Rights are sufficient for both culinary use and for irrigation.

Please note that this Water Rights inventory includes those water rights that are currently being delivered by Zion National Park to the Town, and water rights associated with sources that may or may not be able to deliver the quantity shown on the right to the culinary system.

Figure III.C.1 on the following page shows the existing water rights compared to the projected requirements for year 2028 under historic use. Buildout projections would require 368 gpm and 594 ac-ft at a date projected beyond the year 2028. Should water rights in the irrigation or culinary systems become an issue in the future, consideration to transfer right from points of less production to more could be considered. Care should also be taken that current water rights are protected so they will be available in the future when needed.

Finally, it is recommended that the Town require all new development to provide water rights sufficient for their needs.

**Chart III.C.1 Existing Water Rights vs. Projected Requirements based on Historic Usage**



**WATER RIGHTS SUMMARY**

**Existing Surplus (2008)** = **987 ac-ft**

**Project Surplus FY 2028 (Indoor)** = **822 ac-ft**

**Buildout Surplus (Indoor)** = **589 ac-ft**

**Recommendations**

1. Continue to encourage water conservation through water rates, education and expansion of the secondary water system.
2. Further investigate secondary water system to determine long term water right needs and insure protection of water rights held by the Town.
3. Review water rights at least every 5 years.
4. All new developments bring water rights to the Town.

## SECTION IV WATER SOURCE CAPACITY ANALYSIS

### A. EXISTING WATER SOURCE CAPACITY

To analyze source capacity, all available culinary water sources are first identified and listed in the Table IV.A.1 below.

**Table IV.A.1 Town of Springdale Water Sources**

Town of Springdale Sources (Total flow shown reflects water used in the culinary system)	Total Flow
	gpm
Spring above ZNP Campground	0
Birch Springs East - West of ZNP Museum	0
Birch Springs West - West of ZNP Museum	0
Hummingbird Well	0
Cemetery Well	0
Big Springs	0
North Fork of the Virgin River (Treatment Plant)	400
<b>Source Total =</b>	<b>400</b>

Three source springs listed first in Table IV.A.1 are located in Zion National Park. Water from each of these springs is collected and treated by Zion National Park. In order for the Town to access this water the Town must pay a fee to Zion National Park for any use. For said reasons this water has been considered as an emergency source and would only be used under emergency conditions and not to support future growth, thus source capacity of these springs are shown as 0 gpm.

Springdale's source situation is unique in that of the seven listed sources, three, the Virgin River, Hummingbird Well, and Big Springs are

connected directly to the Town's irrigation system which is shared with the Springdale Consolidated Irrigation Company. Water from these three sources can be pumped to an irrigation pond from which the Town's treatment plant can draw water to be treated and then introduced to the culinary water system. Of the three sources listed which can easily be introduced into the culinary system, the Virgin River is considered the primary source and hence the 400 gpm capacity of the treatment plant is shown as source from the Virgin River.

The Cemetery Well is currently not connected to the culinary or irrigation systems. If extensive work were done and treatment designed, the Cemetery Well could be connected to either system. For these reasons source capacity of the Cemetery Well is shown to be 0 gpm.

From a water rights and physical point of diversion stand point, the Town of Springdale does have multiple sources to supply the Town's culinary system with water. Due to system logistics, locality of some of those rights and the interconnected irrigation system, true source capacity must be reported as the amount of water that can physically be introduced to the culinary water system. The bottleneck in the existing Springdale culinary water system is the treatment plant with a limited capacity of 400 gpm. While the Town can obtain water from multiple other sources, and those capacities can be reasonably assumed from the water rights associated with those points of diversion or measured flow at each source, 400 gpm remains the limit of current source capacity and the number which will be used to determine current and future needs of the system.

### B. EXISTING REQUIRED WATER SOURCE CAPACITY

State of Utah Public Drinking Water Regulations, Section 5, states that a community

should have an adequate water source capacity to physically meet the anticipated peak day demand. The regulations also require the source to be capable of meeting peak irrigation



demands, where no secondary source of irrigation water is available. The Town of Springdale has made irrigation water available as a secondary source to residents and commercial users in the Town and thus this plan will address the source capacity required for indoor use only.

State regulations outline that peak day demand for source capacity requirement per connection should be double the average amount of water required per connection per day. This master plan assumes that the peak day demand in Springdale for source capacity is double the average requirement per ERU based on historic use. This required source capacity per ERU in the Town of Springdale is assumed to be 500 gallons per day for historical use. The required existing source capacity is calculated below:

**Existing Required Source Capacity (FY 2008):  
Historic Usage**

$$699 \text{ ERUs} \times \frac{500 \text{ gpd}}{\text{ERU}} \times \frac{1\text{day}}{1,440 \text{ min.}} = 243 \text{ gpm}$$

The existing source capacity surplus or deficit is determined by subtracting the existing required source capacity of 243 gpm from the total

available source capacity of 400 gpm, which yields an existing surplus of 157 gpm.

**C. PROJECTED REQUIRED WATER SOURCE CAPACITY**

Projected required water source capacity at the end of the planning period is determined from the same information and calculations explained in Part B, except the projected number of culinary water ERUs are substituted in the calculations for the projected number of ERUs. In this situation we consider the 20 year projection under historic usage, as well as the buildup under historic usage. The number of ERUs for the 20 year projection under historic usage is 1,289, while 2,120 is the calculated ERUs for buildup under historic usage. The peak flows remain the same for historic usage at



500 gpd/ERU. The following are calculations for water source capacity requirements for both scenarios.

**Projected Required Source Capacity (FY 2028):  
Historic Usage**

$$1,289 \text{ ERUs} \times \frac{500 \text{ gpd}}{\text{ERU}} \times \frac{1\text{day}}{1,440 \text{ min.}} = 448 \text{ gpm}$$

The projected source capacity surplus or deficit is determined by subtracting the projected required source capacity of 448 gpm from the

total available source capacity of 400 gpm, which yields a projected shortage of 48 gpm at the end of the 20 year planning period using historic use.

**PROJECTED SHORTAGE = (48) gpm**

Projected Required Source Capacity (Buildout):  
Historic Usage

$$2,120 \text{ ERUs} \times \frac{500 \text{ gpd}}{\text{ERU}} \times \frac{1 \text{ day}}{1,440 \text{ min.}} = 736 \text{ gpm}$$

The projected source capacity requirement for historic use results in a projected shortage of 336 gpm for buildout conditions.

**PROJECTED SHORTAGE = (336) gpm**

**D. RECOMMENDED WATER SOURCE IMPROVEMENTS**

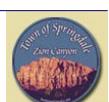
The existing source capacity of 400 gpm is projected to not provide sufficient source capacity through the 20 year planning period and hence not through the projected buildout period either. Using growth projections outlined in section II and historic average use and increase in source capacity is anticipated to be needed by the year 2024.

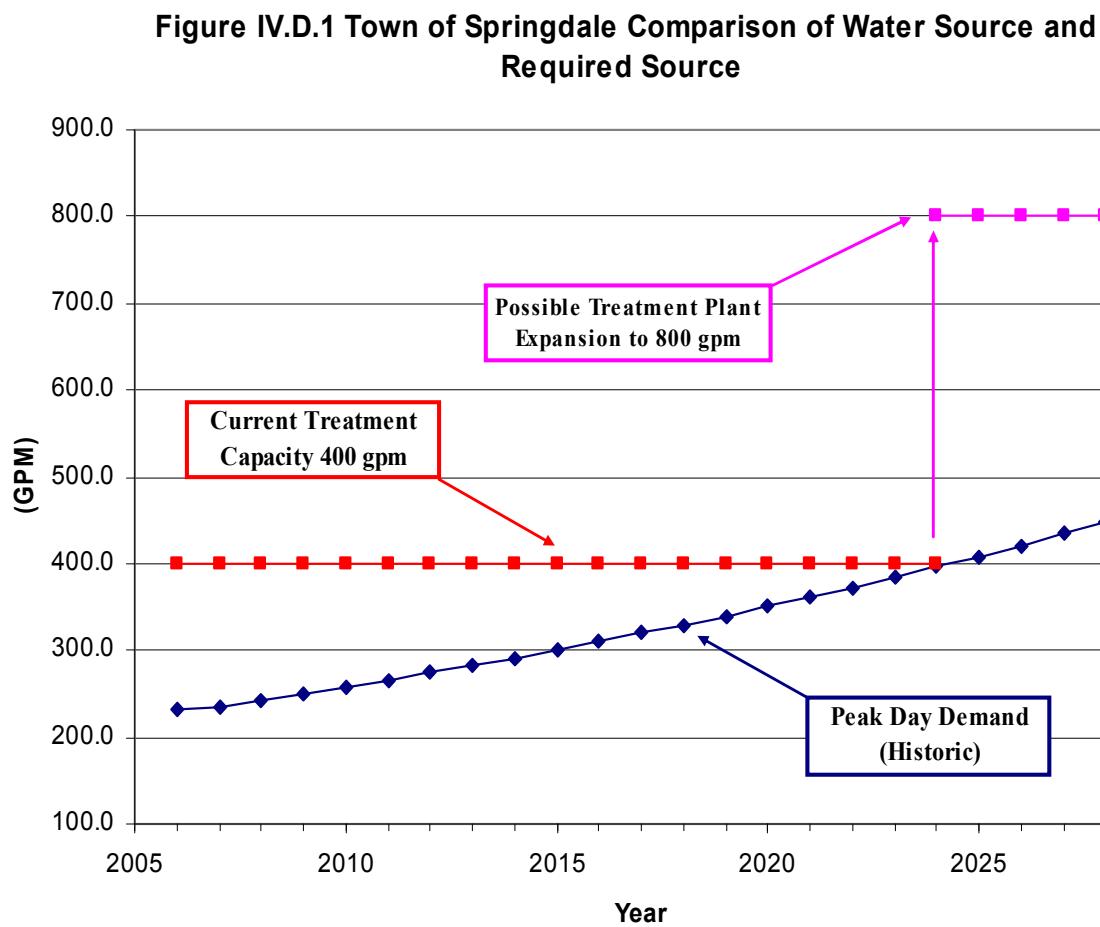
While the Town could address the needed increase in capacity in several different ways, likely the most cost effective option will be to increase the capacity of the treatment plant.

It is anticipated that doubling the size of the existing treatment plant will address the projected source capacity shortages shown to affect the Town in approximately 16 years. Doubling the capacity of the treatment plant could also incorporate a second redundant train for water through the treatment plant should one track ever become contaminated or need to shut down for any reason.

The recommended improvement to double the size of the existing treatment plant will not be included in the project proposed in conjunction with this master plan until further decisions are made in regards to the existing irrigation system, current wells, connection to Zion National Park, and water rights, each of which could affect the proposed treatment plant upgrade.

Figure IV.D.1 graphically represents a comparison between the current water source capacity and the projected required peak day demand flows over the planning period. A depiction of a possible treatment capacity upgrade of 400 gpm is also shown in the graph. The proposed 400 gpm upgrade of the existing treatment plant to 800 gpm capacity is anticipated to meet the buildout source capacity required using historic usage.





**WATER SOURCE SUMMARY**

<b>Existing Source</b>	= 400 gpm
<b>Existing Surplus (2008)</b>	= 157 gpm
<b>Project Shortage FY 2028 (Indoor)</b>	= <b>(-48)</b> gpm
<b>Buildout Shortage (Indoor)</b>	= <b>(-336)</b> gpm

**Recommendations**

1. The Town currently has enough water source capacity for their needs.
2. The Town will not have enough water source capacity under 20-year period or under buildout scenarios.
3. It is suggested that options to increase source capacity be evaluated over the next 5 years.
4. It is suggested that sources such as wells be used at least periodically and flows monitored to monitor and ensure their continual effectiveness.
5. Continue to encourage water conservation through water rates and education.
6. Review water source requirements at least every 5 years.

## SECTION V WATER STORAGE CAPACITY ANALYSIS

### A. EXISTING WATER STORAGE CAPACITY

The Town of Springdale's existing culinary water storage capacity is identified below.

**Table V.A.1 Springdale Water Storage**

Water Storage Unit:	Capacity (gal.)
North Concrete Tank	500,000
East Concrete Tank *	250,000
Anasazi Steel Tank	200,000
<b>Total Existing Storage Capacity =</b>	<b>950,000</b>

\* This Master Plan will show that the East Concrete Tank will be rendered unusable due to the need to construct a new tank at the same elevation as the Anasazi Tank in order to meet pressure/elevation requirements. This will reduce total storage by 250,000 gallons in future projections.

### B. EXISTING REQUIRED WATER STORAGE CAPACITY

Water storage capacity requirements are found in the State of Utah Public Drinking Water Regulations. These regulations require storage for a community's culinary water system to meet one full days use requirement for all connections in the community plus the required fire flows specific to the Town under study plus emergency storage deemed necessary to meet demands in an emergency situation such as a line break or treatment plant failure.

As shown in previous sections, historic average use per ERU in the Town of Springdale is assumed to be 250 gallons per day. Storage requirements for fire protection vary from community to community. In general, fire flow requirements are set by the local Fire Chief or are based on building size and type of

construction. The Town of Springdale has multiple commercial buildings of varying size which require varying fire flows. New improvements in some portions of the Town have been required to install fire suppression systems while other portions of the Town are protected with traditional fire hydrants and no



other special requirements are mandated. The largest structures in the traditional fire protection area have been rated to require at least 3,500 gpm for a period of at least 3 hours. The following storage design calculations will base required fire flow protection on this amount.

Also included in required storage is emergency storage. The Town of Springdale has indicated that for planning purposes the required emergency storage should be on the order of 25% of the total required storage from equalization and fire protection storage. This amount is based on the degree of the water system's reliability and the Town's need for supply redundancy for any and all possible water treatment facility interruptions. Based on the above data and the two separate usage scenarios, the Town of Springdale's storage capacity is calculated below.

Existing Required Storage (FY 2008):

#### Historic Usage

Storage for Average Usage per ERU:

$$699 \text{ ERUs} \times \frac{250 \text{ gpd}}{\text{ERU}} = 174,815 \text{ gallons}$$

Storage for Fire Protection:

$$3,500 \text{ gpm} \times \frac{60 \text{ min}}{\text{hour}} \times 3 \text{ hours} = 630,000 \text{ gallons}$$

Storage for Emergency Supply:

$$0.25 \times (804,815 \text{ gallons}) = 201,204 \text{ gallons}$$

**TOTAL EXISTING REQUIRED  
STORAGE (Historic usage) = 1,006,019  
gallons**

The existing water storage capacity surplus or deficit is determined by subtracting the existing required water storage capacity of 1,006,019 gallons from the total available water storage capacity of 700,000 gallons, which yields an existing shortage of (-306,019) gallons.

**EXISTING STORAGE CAPACITY  
Shortage = (-306,019) gallons**

**C. PROJECTED REQUIRED WATER  
STORAGE CAPACITY**

Projected required culinary water storage capacity at the end of the 20 year planning period is determined from the same factors explained in part B above, but the projected number of culinary water ERUs is inserted into the calculations.

When projecting required water storage capacity the Town of Springdale requested a couple different factors to be taken into consideration in the projection analysis. First, since the need for a new treatment facility is roughly 16 years in the future, new storage suggested in this Master Plan will also be used to increase system dependability in time of water treatment facility failures or scheduled maintenances (emergency storage). Second, the Town desires not to build additional storage in the future, therefore the buildout scenario will

be heavily weighed in sizing the new storage tank.

As with water rights and source capacity, storage capacity will be analyzed for the 20 year growth and buildout projections under historic usage. As before, fire suppression storage will be based on 3,500 gpm for three hours. The following are calculations for water storage requirements for both scenarios.

Projected Required Storage Capacity (FY 2028):

**Historic Usage**

Storage for Average Usage per ERU:

$$1,289 \text{ ERUs} \times \frac{250 \text{ gpd}}{\text{ERU}} = 322,246 \text{ gallons}$$

Storage for Fire Protection:

$$3,500 \text{ gpm} \times \frac{60 \text{ min}}{\text{hour}} \times 3 \text{ hours} = 630,000 \text{ gallons}$$

Storage for Emergency Supply:

$$0.25 \times (952,246 \text{ gallons}) = 238,062 \text{ gallons}$$

**TOTAL PROJECTED REQUIRED  
STORAGE = 1,190,308 gallons**

The projected water storage capacity surplus or deficit is determined by subtracting the projected required water storage capacity of 1,190,308 gallons from the total available water storage capacity of 700,000 gallons, which yields a projected shortage of 490,308 gallons at the end of the 20 year planning period.

**PROJECTED 2028 WATER STORAGE  
CAPACITY SHORTAGE (Historic usage) =  
(490,308) gallons**

Projected Required Storage Capacity (Buildout):

**Historic Usage**

Storage for Average Usage per ERU:

$$2,120 \text{ ERUs} \times \frac{250 \text{ gpd}}{\text{ERU}} = 530,000 \text{ gallons}$$

Storage for Fire Protection:

$$3,500 \text{ gpm} \times \frac{60 \text{ min}}{\text{hour}} \times 3 \text{ hours} = 630,000 \text{ gallons}$$

Storage for Emergency Supply:

$$0.25 \times (1,160,000 \text{ gallons}) = 290,000 \text{ gallons}$$

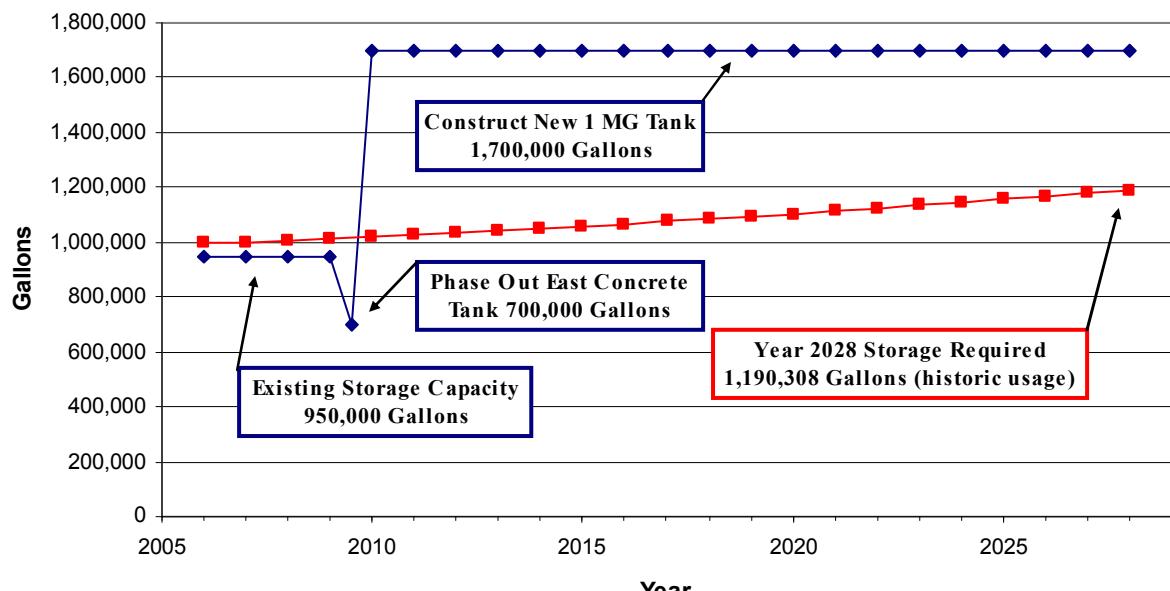
**TOTAL PROJECTED REQUIRED  
STORAGE = 1,450,000 gallons**

The projected water storage capacity, calculated as before, yields a projected shortage of 750,000 gallons at the end of the buildout period.

**PROJECTED (Buildout) WATER  
STORAGE CAPACITY SHORTAGE  
(Historic usage) = (750,000) gallons**

Figure V.C.1 shows the timeline of projected storage required versus current storage capacity using historic usage.

**Figure V.C.1 Town of Springdale Water Storage (3500 gpm Fire Flow x 3 hours)**



## D. RECOMMENDED WATER STORAGE CAPACITY IMPROVEMENTS

The existing required storage capacity calculations yield a shortage of storage capacity by 306,019 gallons with historic usage when the lower elevation East Concrete tank (250,000 gallons) is not included in the total available storage. Since the Town of Springdale desires not to build additional tanks in the future, and this appears to be logically and financially possible, it is recommended that the buildout scenario be used instead of the 20 year planning period to determine projected required storage capacity. The buildout required storage capacity calculations result in a shortage of 750,000 gallons with historic usage.

In addition to running an analysis using historic usage, the Town of Springdale requested that analysis for storage capacity be run using the State Standard usage as well. These calculations are similar to the calculations previously found in this Section except State Standard usage is 400 gpd instead of the 250 gpd for historic usage. The calculations for the storage capacity using State Standard usage can be found in Appendix B. These calculations show that by State Standard usage the Town's future storage capacity deficit at buildout is 1,147,500 gallons. Therefore, based on the buildout scenario using State Standard and historic usage, the recommended size of the storage tank to be constructed is 1 million gallons, a value which lies between the historic and State Standard usage scenarios. Using State Standard usage along with historic provides a more conservative estimate ensuring more confidence that the Town will not need to build any additional storage tanks in the future.

This tank will also meet the goals of the Town by providing enough required storage to handle all the demands up to and including anticipated buildout, thus negating the requirement of

constructing another storage tank in the future. In addition, the new storage tank will provide enough emergency storage to accommodate facility maintenance or possible treatment interruptions until a new or upgraded treatment facility would be required to meet source requirements.

### WATER STORAGE SUMMARY

<b>Existing Shortage</b>	<b>= (306,019)</b>	<b>Gallons</b>
<b>Project Shortage (FY 2028)</b>	<b>= (490,308)</b>	<b>Gallons</b>
<b>Project Shortage (Buildout)</b>	<b>= (750,000)</b>	<b>Gallons</b>

#### Recommendations

- 1. Recognize the immediate need for new storage facilities to accommodate current needs of the Town.**
- 2. Construct a tank large enough to supply storage at buildout conditions.**
- 3. Continue to encourage irrigation system use decreasing culinary water use.**
- 4. Continue to encourage water conservation through water rates and education.**
- 5. Review water storage requirements at least every 5 years.**
- 6. Consider use of the East Concrete Tank for secondary irrigation purposes.**

### SECTION VI WATER TREATMENT REQUIREMENTS

#### A. GENERAL REQUIREMENTS

The State of Utah Public Drinking Water Regulations, in accordance with the National Safe Drinking Water Act, have adopted “primary” regulations for the protection of public health, and “secondary” regulations related to taste and aesthetics. The regulations recommend that all culinary water sources have provisions for continuous disinfection.

#### B. EXISTING TREATMENT FACILITIES

The Town of Springdale has a treatment facility to treat all the water sources used by the Town. Water from the Virgin River and all other sources are pumped to a 2 million gallon holding pond where the water goes through initial settling. The water for treatment is then sent into the primary treating facility compound. First, alum sulfate is injected into the water to act as a coagulant. The water is then monitored for turbidity and sent into the 74,500 gallon flocculation basin also known as the Contraflow. Once the water passes through the Contraflow it is sent into four separate sand and activated carbon filters. Each filter has a flow capacity of 100 gpm. This process is the limiting process of the treatment plant, thus limiting the maximum treatment flow to 400 gpm. The water is then sent to a clear well for collection and residual disinfection takes place by method of chlorine gas. From the clear well the water is pumped to the 500,000 gallon tank for distribution. Currently this treatment system has one train or path which water can follow for treatment. The State would like to see multiple trains in the Town’s water treatment system in case of contamination or other unforeseen shutdowns. Currently all redundancy lies in the extra storage capacity,

thus providing the needed water in case of emergency shutdown of the treatment system. It is recommended that any future upgrades or additional treatment facilities built should be designed to add multiple trains to accommodate the suggested redundancy.



#### C. RECOMMENDED WATER TREATMENT FACILITY IMPROVEMENTS

The current treatment facility is capable of producing the amount of treated water required by the Town’s needs. As suggested in Section IV, the needs of the Town will exceed the current capacity of the treatment facility likely around the year 2024 based on historical use data. Once this takes place the Town will need to provide additional capacity to the treatment facility by either expansion of the current facility or by building a second treatment facility. It is suggested that water demands be revised every 5 years or less to ensure that current needs are provided for by the capacity of the treatment facility.

## SECTION VII

### WATER DISTRIBUTION SYSTEM ANALYSIS

#### A. EXISTING DISTRIBUTION SYSTEM ANALYSIS

The State of Utah Administrative Rules for Public Drinking Water Systems, R309-105-9, requires that no connection experience less than 20 psi at any time during operation of the system. The regulations also require that the distribution system be sized to maintain 20 psi during peak day conditions with fire flow demands, 30 psi during conditions of peak instantaneous demand, and 40 psi during peak day demand.



As previously discussed in Section V, the Town of Springdale has identified 3,500 gpm as a goal for fire flow demands, but for the distribution system analysis the 3,500 gpm flow is not required for everywhere in the system. Currently, the Town has two structures which were built before the requirement that all rooms in large structures be equipped with sprinklers. These structures are the only locations in the system where the fire flows are required to be 3,500 gpm for traditional fire hydrant fire protection. Elsewhere in the system, fire flows are checked for compliance with R309-105-9.

As stated in the source capacity analysis, peak day flows are equal to twice the average day flow, while the equation to calculate the peak

instantaneous demand is found in the State of Utah Administrative Rules for Public Drinking Water Systems rule R309-105-9. This equation can also be found below.

Existing Peak Day Demand (FY 2008):

#### Indoor Use (Historic)

$$250 \text{ gpd/ERU} \times 699 \text{ ERU} / 1,440 \text{ min/day} = 121 \text{ gpm}$$

$$Q_{\text{Peak Day}} = 121 \times 2 = 242 \text{ gpm}$$

#### Existing Total Peak Day Demand:

$$Q_{\text{Total Peak Day}} = Q_{\text{Peak Day}} + Q_{\text{Fire Flow}} = \underline{\underline{3,742 \text{ gpm}}}$$

Existing Peak Instantaneous Demand:

$$Q_{\text{Peak Instantaneous (Indoor)}} = 10.8 \times N^{0.64}$$

(N = Number of ERUs)

$$= 10.8 \times (699)^{0.64} = \underline{\underline{714 \text{ gpm}}}$$

The resulting peaking factor is determined by dividing the existing peak instantaneous demand by the average day demand.

$$\text{Peaking Factor (Historical)} = 5.9$$

The Town's culinary water distribution system has been modeled, using the computer program H2ONet® by MWHSoft. For the existing network under peak instantaneous demands there are several areas with pressures in the low 30s psi which does supply the required 30 psi, but is near insufficient. Overall the system seems to be providing good service to all other connections. Analysis of required fireflows under peak day demand, shows that there are areas that the existing system cannot produce the minimum required 1,000 gpm flows and some areas even produce flows less than 100 gpm. Mainly these areas contain undersized main lines (none of these locations have existing fire hydrants). The system also has areas that drop below the required 20 psi and in some instances pressures drop to negative.

Even though 1,000 gpm cannot be supplied everywhere in the system, the two structures



requiring 3,500 gpm are supplied with their required flows. 3,500 gpm flows are only obtained for these structures in select fire hydrants along Zion Park Boulevard, and these flows cannot be achieved off Zion Park Boulevard from the hydrants behind the large structures. A map of the existing system is provided in Appendix C.

## B. PROJECTED DISTRIBUTION SYSTEM ANALYSIS

The projected distribution system analysis is performed using the same assumptions as used in the existing system analysis, except that the projected number of connections for year 2028 or under buildout conditions are inserted into the calculations. Two different scenarios will be explored by this analysis which include the following; a 20 year projection using historic usage, as well as buildout conditions, again using historic usage.

### Projected Peak Day Demand (FY 2028): Indoor Use (Historic)

250 gpd/ERU X 1,289 ERU / 1,440 min/day =  
**224 gpm**  
 $Q_{Peak\ Day} = 224 \times 2 = 448 \text{ gpm}$

### Projected Total Peak Demand:

$Q_{Total\ Peak\ Day} = Q_{Peak\ Day} + Q_{Fire\ Flow} = \underline{\underline{3,948\ gpm}}$   
Projected Peak Instantaneous Demand:

$$Q_{Peak\ Instantaneous\ (Indoor)} = 10.8 \times N^{0.64}$$

(N = Number of ERUs)

$$= 10.8 \times (1,289)^{0.64} = \underline{\underline{1,056\ gpm}}$$

### Projected Peak Day Demand (Buildout): Indoor Use (Historic)

250 gpd/ERU X 2,120 ERU / 1,440 min/day =  
**368 gpm**  
 $Q_{Peak\ Day} = 368 \times 2 = 736 \text{ gpm}$

### Projected Total Peak Demand:

$Q_{Total\ Peak\ Day} = Q_{Peak\ Day} + Q_{Fire\ Flow} = \underline{\underline{4,236\ gpm}}$

### Projected Peak Instantaneous Demand:

$$Q_{Peak\ Instantaneous\ (Indoor)} = 10.8 \times N^{0.64}$$

(N = Number of ERUs)

$$= 10.8 \times (2,120)^{0.64} = \underline{\underline{1,453\ gpm}}$$

Using each of the above listed flows the water system model was used to analyze the existing culinary water system. Results from the model can be found in the appendices. From deficiencies observed in the analysis specific improvements are recommended which will remedy problems in deficient areas.

## C. RECOMMENDED DISTRIBUTION SYSTEM IMPROVEMENTS

To meet the requirements set forth by the State of Utah Administrative Rules for Public Drinking Water Systems, R309-105-9, as previously mentioned, the Town of Springdale is in need of pipeline replacements and upsizing. Not only are these improvements suggested for existing system needs but also for future system needs.

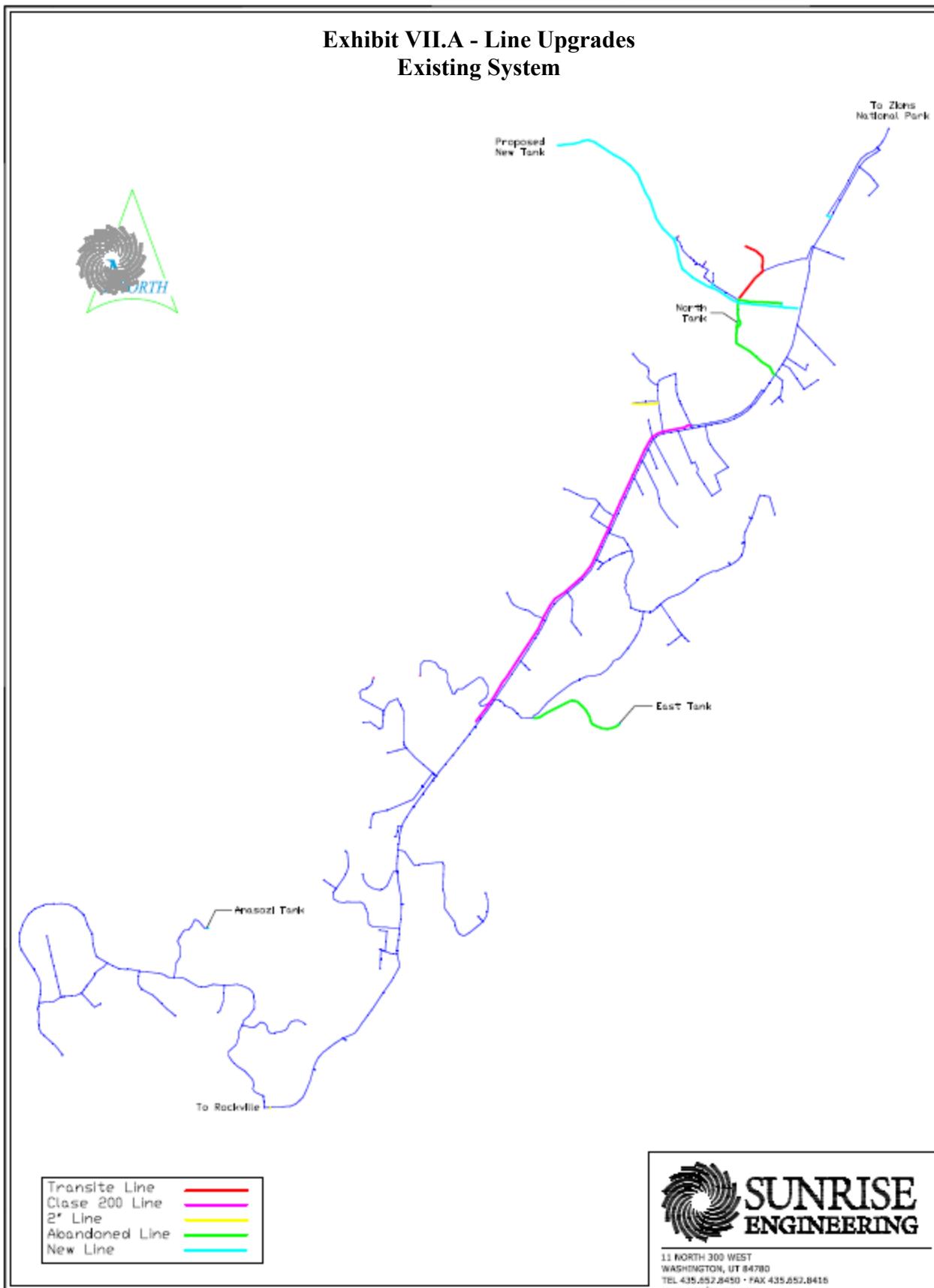
Following the analysis done in H2ONet® by MWHSoft, the following distribution system improvements are being recommended.

- ◆ An 8 inch line will replace the 2 inch line located along Foothill Lane running from Winderland Lane West to the end of Foothill Lane.
- ◆ A 10 inch PVC line will need to replace the current line along Zion Park Boulevard from approximately Paradise Road to Quail Ridge Road.
- ◆ A 12 inch line will need to be installed from the location of the new tank, located approximately near the top of Lion Boulevard, down Lion Boulevard and connecting into the line along Zion Park Boulevard.
- ◆ A 10 inch PVC line will also need to be installed tying into the new 12 inch line where Stone Mountain Road and Lion Boulevard intersect, across the field to where Paradise Road and Winderland Lane intersect, and down Paradise Road to Zion Park Boulevard. It should also be noted that this section of line is part of a development and not part of the recommended improvements.
- ◆ Additionally, due to government regulations on Transite lines used in culinary systems, an 8 inch PVC line will need to replace an old Transite line running North East from Lion Boulevard along Stone Mountain Road and up Stone Mountain Road as it heads North West.

Running the water model analysis with the proposed improvements shows the Town would be in compliance with State rules previously mentioned. Exhibit VII.A shows the location of the current lines that are to be replaced by the recommended improvements. It is also recommended that the Town keep record of dated and leaky lines that need to be replaced

and create a renewal and replacement fund to pay for replacement of trouble lines as deemed necessary. Every 5 years or less the distribution system should be reanalyzed to insure it is compliant with up to date rules and regulations, this can be accomplished through an updated water master plan or similar analysis.

**Exhibit VII.A - Line Upgrades  
Existing System**




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# SECTION VIII

## SUMMARY OF RECOMMENDED CULINARY WATER SYSTEM IMPROVEMENTS

### A. RECOMMENDED IMPROVEMENTS

Based on the findings from Sections III - VII, showing requirements for growth projected over the next 20 years and buildout, it is recommended that the Town proceed to implement the following recommended improvements in anticipation of increased system demands due to new growth. After modeling the system for projected flows over the next 20 years and under buildout conditions, the following summarizes the improvements recommended for the various aspects of the culinary water system.

#### **Water Rights**

1. The Town currently has enough water rights to service the Town, as well as through a 20 year planning period and buildout period.
2. Ensure current trends of outdoor water usage of irrigational water stay intact. Review with regard to secondary irrigation system and associated water rights.
3. Require developers to provide water rights.

#### **Water Storage Capacity**

1. Construct an additional 1 Million Gallon Concrete Storage Tank to accommodate expected buildout of the Town as well as provide emergency storage as a back up to treatment facility shutdowns.
2. Ensure additional water storage tank is located at high enough elevation to accommodate any foreseen developments.

#### **Water Treatment**

1. As the Town grows, an additional treatment facility upgrade will be required.
2. Ensure future design of treatment plant incorporates multiple trains for redundancy.

#### **Water Source Capacity**

1. The Town currently has enough water source capacity for their needs, but not enough through the 20 year planning and buildout periods. It is suggested that an evaluation be made at a later date since the shortage of capacity is expected sometime near the year 2024.
2. Consider expansion of treatment capacity to provide redundancy of source and to meet future demands.
3. It is also suggested that sources such as wells be monitored to ensure their continual effectiveness and to demonstrate beneficial use as it relates to water right requirements.

#### **Distribution System**

1. Replace 8" transit line with 8" PVC C900 pipe.
2. Replace existing 2" line in Foothill Lane with new 8" PVC C900 pipe.
3. Construct 10" line on Paradise Road and Zion Park Boulevard.
4. Construct 12" line from the new tank location to the highway.
5. Monitor lines that need to be replaced and put funds aside in a renewal and replacement fund to pay for outdated lines as seen fit.

### B. ENGINEER'S OPINION OF PROBABLE COST

Engineer's opinions of probable costs for the recommended culinary water improvements are provided in Table VIII.B.1.

Included in the Opinions of Probable Cost for the proposed projects are anticipated construction costs, a contingency budget, and a budget for other normal project costs such as survey, administration, engineering, legal services, fiscal costs, rights-of-way and etc. **Please note that the date of the Opinion of Probable Cost is May 2008.**

This master plan is a 20-year plan, designed to consider the projected growth and required demands for the Town's culinary water system over the next 20 years. In addition to the 20-year plan, the Town of Springdale has asked that the demands under buildup be considered. The proposed improvements have taken into consideration both the 20-year plan, the buildup scenario and other desires of the Town. A review of the locations of the proposed improvements can be found on Exhibit VII.A, which can be found in Section VII. This exhibit shows the locations of the current water lines for replacement as well as the preliminary proposed placement of the 1 million gallon water tank.

### C. PROPOSED FINANCING PLAN

Table VIII.C.1 outlines a sample financing plan for the recommended improvements. The total proposed cost for the financing plan is \$2,968,600. This cost is split 69% / 31% into a DWB Loan and a DWB Grant. The financing plan assumes that the loan has an interest rate of 1% and payback term of 30 years. The financing plan also considers the expected first year expenses including salaries, utilities, legal and professional fees, as well as the existing debt service.

For the Town to pay for these expenses they would need to address impact fees and the monthly water user fee. The proposed amount for these fees can be found in Table VII.C.1. Section X will go into greater detail on how the impact fees were determined. The average monthly water user fee was calculated using the sample financing plan by taking all the expected expenditures and existing debt service and subtracting off the total impact fees and other expected revenues obtained that year. The amount was then divided by the number of expected ERUs in the system that year to come up with the average monthly water user fee per ERU. Then Section IX lays out possible water rate structures set to cover the average monthly water user fees.

The last portion of the financing plan accounts for the user's additional water fees from irrigation, which should be considered when totaling the user's water fees. Finally, the cash flow spreadsheet can be found in Appendix E, which implements the proposed financing plan over the next 20 year planning period. Included in the cash flow spreadsheet are anticipated projects, such as the water treatment facility which needs to be implemented into the current system within the next 16 years according to current growth trends.

**TABLE VIII.B.1**  
**Town of Springdale**  
**Engineer's Opinion of Probable Cost**  
**Install New 1.0 Million Gallon Tank, In Town Replacements**

12-May-08  
 ALA

NO.	DESCRIPTION	Estimated Quantity	Units	Unit Price	TOTAL COST
1	Mobilization	1	LS	\$ 96,000.00	\$ 96,000.00
2	Earthwork 1.0 Million Gallon Tank	1	LS	\$ 85,000.00	\$ 85,000.00
3	Construct 1.0 Million Gallon Tank	1	LS	\$ 800,000.00	\$ 800,000.00
4	Tank Appurtenances	1	LS	\$ 75,000.00	\$ 75,000.00
5	Chainlink Fence & Gate	800	LN.FT.	\$ 22.00	\$ 17,600.00
6	Metering Station	1	LS	\$ 25,000.00	\$ 25,000.00
7	8" PVC Line & Fittings, Replace transite pipe, Foot Hill Lane	1,750	LN.FT.	\$ 21.00	\$ 36,750.00
8	8" Gate Valve Assembly	18	Each	\$ 1,200.00	\$ 21,600.00
9	Fire Hydrant Reconnection	16	Each	\$ 2,000.00	\$ 32,000.00
10	Fire Hydrant Assembly	4	EA.	\$ 3,500.00	\$ 14,000.00
11	10" PVC Line & Fittings, Paradise and Zion Park Boulevard	7,750	LN.FT.	\$ 27.00	\$ 209,250.00
12	10" Gate Valve Assembly	19	Each	\$ 1,800.00	\$ 34,200.00
13	12" PVC Line & Fittings, New Tank to Highway	5,250	LN.FT.	\$ 33.00	\$ 173,250.00
14	12" Gate Valve Assembly	5	EA.	\$ 2,200.00	\$ 11,000.00
15	Untreated Base Course (6" Depth in Trench)	91,163	SQ. FT.	\$ 0.85	\$ 77,488.55
16	Bituminous Surface Course	91,163	SQ. FT.	\$ 2.15	\$ 196,000.45
17	Reconnection of Meters (saddle, meter setter & service lateral pipe)	37	Each	\$ 900.00	\$ 33,300.00
18	Retrofitting the existing booster pumps to pump to the new tank	1	LS	\$ 93,000.00	\$ 93,000.00
19					
20	<b>Sub-Total</b>				\$ 2,030,400
21	<b>Contingency</b>	14%			\$ 284,300
22	<b>Total Construction</b>				<b>\$ 2,314,700</b>
23	<b>INCIDENTALS</b>				
24	Funding & Administrative Services	1%	L.S.		\$ 23,100
25	Legal and Fiscal		Est.		\$ 69,800
26	Engineering Design	5.3%	L.S.		\$ 156,000
27	Engineering Construction Services	5.5%	Hourly		\$ 185,200
28	SWPPP (Storm Water Pollution Protection Plan)		Est.		\$ -
29	Environmental/Archeology		Est.		\$ 20,000
30	Geotechnical Engineering		Est.		\$ 30,000
31	Electrical Engineering		Est.		\$ 15,000
32	Land & R/W Acquisition/Negotiation		Est.		\$ 20,000
33	Water Rights Research and POD Applications		Est.		\$ 10,000
34	Survey & GIS Mapping		Est.		\$ 20,000
35	Radio Read Meters/Equipment/Software - Materials, no Install		Est.		\$ 40,000
36	SCADA Improvements		Est.		\$ 45,000
37	Miscellaneous Engineering Services		Est.		\$ 20,000
38					
39	<b>TOTAL PROJECT COST</b>				<b>\$ 2,968,800</b>

*In providing opinions of probable construction cost, the Client understands that the Engineer has no control over costs or the price of labor, equipment or materials, or over the Contractor's method of pricing, and that the opinion of probable construction cost provided herein is made on the basis of the Engineer's qualifications and experience. The Engineer makes no warranty, expressed or implied, as to the accuracy of such opinions compared to bid or actual costs.*

<b>TABLE VIII.C.1</b>			
<b>TOWN OF SPRINGDALE</b>			
<b>FY 2009 PROPOSED FINANCING PLAN</b>			
<b>TOTAL PROJECT COST</b>			<b>\$ 2,968,600</b>
<b>FY 2009 EXPENSES</b>			
<b>Proposed Funding:</b>	<b>Rate</b>	<b>Term in Yrs.</b>	<b>Principal</b>
Self Participation			200,000
DWB Grant			768,600
DWB Loan	1.00%	30	2,000,000
<b>TOTAL PROJECT FUNDING:</b>			<b>\$2,968,600</b>
<b>EXPENSES: (First Year of New Debt Serv. Pmt.)</b>			
Salaries, wages and benefits			\$121,911
Office expenses and travel			\$6,584
Repairs and maintenance			\$39,526
Utilities			\$14,370
Legal and professional fees			\$12,761
Garbage collection			\$0
Contract services			\$2,442
Insurance			\$10,739
Miscellaneous			\$15,235
Depreciation			\$0
<b>Subtotal Expenses:</b>			<b>\$223,568</b>
<b>EXISTING DEBT SERVICE</b>			
No Interest State Loan	0%		\$17,080
Loan Reserve (Payment /10)			\$1,708
Water Revenue Bond 1995A	0%	20	\$48,000
Bond Reserve (Payment/6)			\$8,000
Water Revenue Bond 2004 Tank Project	1%	20	\$21,260
Bond Reserve (Payment/6)			\$10,000
<b>Subtotal Existing Annual Debt Service:</b>			<b>\$106,048</b>
<b>NEW DEBT SERVICE</b>			
New Loan(s)	100.0%	0	\$77,496
Loan Reserve (Payment/10)			\$7,700
<b>Subtotal New Annual Debt Service:</b>			<b>\$85,196</b>
Renewal and Replacement Fund			\$66,000
<b>GRAND TOTAL EXPENSES:</b>			<b>\$480,812</b>
<b>ANNUAL INCOME</b>			
Transient Room Tax Revenues			\$60,000
Projected Yearly Impact Fees Received			\$70,201
Total Number Of ERUs			721
Average Monthly Water User Rate/ERU			\$40.52
<b>TOTAL ANNUAL INCOME:</b>			<b>\$480,812</b>
Average Monthly Irrigation Water User Rate/ERU			\$4.88
Total Average Monthly Water Cost/ERU			\$45.40

## SECTION IX

### WATER RATE ANALYSIS

#### A. GENERAL

Generally water rates are a combination of base rates and overage rates. Typically, a base amount of water is provided for the base rate charge. The base rate is charged to all connections in the system whether or not water is used. Overage rates are normally set to encourage water conservation. The Town has adopted the rate structure shown in Table IX.A.1. A separate structure has been established for either residential water use or commercial and other water use.

TABLE IX.A.1		
Town of Springdale		
Existing Residential Water Rate Structure		
Total Base Rate	\$13.46	per ERU/Month
Includes	0	Gallons
Overage Steps		
Cost Per 1,000 Gal.	Low Gallons	High Gallons
\$2.92	0	5,000
\$3.25	5,001	10,000
\$3.59	10,001	25,000
\$3.93	25,001	& UP
Existing Commercial and Other Water Rate Structure		
Total Base Rate	\$26.93	per Connection/Month
Includes	0	Gallons
Overage Steps		
Cost Per 1,000 Gal.	Low Gallons	High Gallons
\$4.49	0	10,000
\$5.05	10,001	20,000
\$5.61	20,001	50,000
\$6.17	50,001	& UP

The overage step structure was established to promote conservation and reward low water

users. The current average water rate has been established to be \$27.18 for residential and \$209.57 for commercial. These figures were determined by applying the average monthly usage of 4,568 gallons for residential and 35,550 gallons for commercial, to the rate structure currently use by the Town.

#### B. AVERAGE RATE DETERMINATION FOR FY 2009

Tables IX.B.1 shows a method used to determine the average water rate per ERU, which should be divided between all system customers. The purpose for considering FY 2009 is to determine the effect of the first full year of the new loans from suggested improvement projects, which is being funded by the Utah Division of Drinking Water Board.

As described in Section II an ERU is defined as a residential connection or a commercial connection using 250 gallons per day by historic usage for indoor use only. If a commercial connection uses 1,000 gallons per day it would then consist of 4 ERUs by historic use.

Annual revenues must be sufficient to cover the expenses incurred by the construction, maintenance, and administration of the water system. These expenses include debt service, utilities, personnel salaries and benefits, system maintenance, legal and professional fees, and other miscellaneous items. It is strongly recommended that the water department maintain a funded depreciation account or a replacement fund to provide the money necessary for replacement and repair of water department facilities and pipelines. The loan for the proposed project may require a fund where at least 5% of the expenses, including debt service, be set aside for this purpose.

In the most recent audit completed to date for the Town, total revenues from user fees for the Springdale Town water account in 2006 were \$299,378. When \$299,378 is divided by the

<b>TABLE IX.B.1</b>					
<b>TOWN OF SPRINGDALE</b>					
<b>FIXED RATE ANALYSIS</b>					
<b>FY 2009 Expenses</b>	<b>Fixed</b>		<b>Variable</b>		<b>Total</b>
Salaries, wages and benefits	\$	85,338	\$	36,573	\$ 121,911
Office expenses and travel	\$	4,609	\$	1,975	\$ 6,584
Repairs and maintenance	\$	27,668	\$	11,858	\$ 39,526
Utilities	\$	5,748	\$	8,622	\$ 14,370
Legal and professional fees	\$	-	\$	12,761	\$ 12,761
Garbage collection	\$	-	\$	-	\$ 0
Contract services	\$	-	\$	2,442	\$ 2,442
Insurance	\$	10,739	\$	-	\$ 10,739
Miscellaneous	\$	6,094	\$	9,141	\$ 15,235
Depreciation	\$	-	\$	-	\$ 0
<b>EXISTING DEBT SERVICE (PARTIALLY IMPACT FEE ELIGIBLE)</b>					
No Interest State Loan	\$	17,080	\$	-	\$ 17,080
Loan Reserve (Payment /10)	\$	1,708	\$	-	\$ 1,708
Water Revenue Bond 1995A	\$	48,000	\$	-	\$ 48,000
Bond Reserve (Payment/6)	\$	8,000	\$	-	\$ 8,000
Water Revenue Bond 2004 Tank Project	\$	21,260	\$	-	\$ 21,260
Bond Reserve (Payment/6)	\$	10,000	\$	-	\$ 10,000
<b>NEW DEBT SERVICE (NOT IMPACT FEE ELIGIBLE)</b>					
New Loan(s)	\$	11,449	\$	-	\$ 11,449
Loan Reserve (Payment/10)	\$	1,138	\$	-	\$ 1,138
Renewal and Replacement Fund	\$	9,751	\$	-	\$ 9,751
<b>NEW DEBT SERVICE (IMPACT FEE ELIGIBLE)</b>					
New Loan	\$	66,047	\$	-	\$ 66,047
Loan Reserve (Payment/10)	\$	6,562	\$	-	\$ 6,562
Renewal and Replacement	\$	56,249	\$	-	\$ 56,249
<b>Total Expenses:</b>	\$	397,439	\$	83,373	<b>\$ 480,812</b>
Transient Room Tax Revenues	\$	60,000			
Impact Fee Income	\$	58,028	\$	12,173	\$ 70,201
Total Expenses - Impact Fees	\$	279,411	\$	71,200	\$ 350,611
Total Projected System ERUs in FY 2009		721		721	721
<b>Monthly Cost/ERU in FY 2009</b>		<u>\$32.29</u>		<u>\$8.23</u>	<b><u>\$40.52</u></b>

estimated total number of ERUs in the system for 2006, which was 677 ERUs, and again by 12 months, the result is \$36.85 per ERU per month. This amount is the average total cost per month that each ERU connected to the system paid (not including any impact fee revenue) in 2006. This value represents how much the total cost per ERU connected to the system would need to pay in order for the water budget to pay for itself with the yearly revenues it would take in. The previous year resulted in a slightly larger average total cost per month at \$37.38.

The Town has set aside \$60,000 a year from the Transient Room Tax to help pay for water fund expenses. By doing so it subsidizes the costs to the Town's customers. Therefore, this \$60,000 was used in the analysis when estimating the new average cost per month per ERU. Based on the calculations shown in Table IX.B.1, the average water rate per residential connection (1ERU) for any newly adopted rate structure for the year 2009 would need to be approximately \$40.52 when taking on new debt. This assumes that the system has 721 ERUs in 2009 when payments on a loan would begin. Note that the proposed funding shows that the average expenses per connection remain near current average rates and that the Town is able to make the financing payments for the proposed improvements

### C. BASE AND OVERAGE RATE DETERMINATION

This study includes separating the average user rate into base and overage rates, and investigates possible rate structures that would promote conservation and work hand-in-hand with drought management policies. In order to determine a base and overage schedule, the projected expenses of the water system for FY 2009 have been separated into fixed and variable expenses (Table IX.B.1). It is recommended that the base rate and any portion paid by tax revenue should cover the fixed

expenses of a system. However, Springdale Town may decide to lower the base rate, and increase the variable costs in order to promote conservation. Table IX.B.1 suggests a possible scenario for determining base and overage rates for Springdale Town. Included in the base rate is \$32.29 for fixed costs and \$8.23 for variable costs. This rate scenario simply identifies base and overage rates that should satisfy the revenue requirements based on estimated operation and maintenance (O&M) expenses and projected water usage. Springdale Town is able to set the rate structure to any amount it deems to be fair. However, the rates should be such that the system remains financially viable.

The Town of Springdale will need to determine a rate schedule / tax revenue package that will result in revenues that will average \$40.52/ ERU/month in order to provide the necessary culinary water system improvements as recommended in this plan and maintain the current level of O&M. The base and overage rates should be examined each year to ensure that enough revenue is being generated to cover the expenses.

### D. POSSIBLE RATE STRUCTURE

Tables IX.D.1 and IX.D.2 illustrate possible rate structures based partially on the base and overage rates suggested in Section C. The overage rate structure is stepped to promote conservation by charging a higher amount for excessive water usage similar to the existing rate structure used by the Town of Springdale. The tables also include some examples of water bills based on the proposed rate structure and show bills based on existing rates for comparison. Transient Room Tax revenue used to pay water fund expenses is included in this analysis. An amount is shown for the average monthly use of approximately 4,700 gallons for residential usage and 35,500 gallons for commercial usage. These figures are based on actual historical usages of 156 gpd for residential and 1,185 gpd for commercial. The

only way to confirm that the average rate produced will cover annual expenses is to implement the structure and evaluate the results after a few years of use.

TABLE IX.D.1					
TOWN OF SPRINGDALE					
Other Possible Water Rate Structures for <u>Residential Water Usage</u>					
Total Base Rate	\$14.74	ERU/Month		Total Base Rate	\$17.52
Includes	0	Gallons		Includes	0
Overage Steps			Overage Steps		
Cost Per 1,000 Gal.	Low Gallons	High Gallons	Cost Per 1,000 Gal.	Low Gallons	High Gallons
\$3.05	0	5,000	\$2.46	0	5,000
\$3.40	5,001	10,000	\$2.76	5,001	10,000
\$3.75	10,001	25,000	\$3.06	10,001	25,000
\$4.10	25,001	& UP	\$3.36	25,001	& UP
Usage (Gallons)	In Town Rates		Usage (Gallons)	In Town Rates	
	New Rate	Old Rate		New Rate	Old Rate
0	\$ 14.74	\$ 13.46	0	\$ 17.52	\$ 13.46
3,000	\$ 23.89	\$ 22.22	3,000	\$ 24.90	\$ 22.22
4,700	\$ 29.08	\$ 27.18	4,700	\$ 29.08	\$ 27.18
10,000	\$ 46.99	\$ 44.31	10,000	\$ 43.62	\$ 44.31
35,500	\$ 146.29	\$ 139.43	35,500	\$ 124.80	\$ 139.43

TABLE IX.D.2					
TOWN OF SPRINGDALE					
Other Possible Water Rate Structures for <u>Commercial Water Usage</u>					
Total Base Rate	\$31.45	ERU/Month		Total Base Rate	\$35.00
Includes	0	Gallons		Includes	0
Overage Steps			Overage Steps		
Cost Per 1,000 Gal.	Low Gallons	High Gallons	Cost Per 1,000 Gal.	Low Gallons	High Gallons
\$4.78	0	10,000	\$4.68	0	10,000
\$5.38	10,001	20,000	\$5.28	10,001	20,000
\$5.98	20,001	50,000	\$5.88	20,001	50,000
\$6.58	50,001	& UP	\$6.48	50,001	& UP
Usage (Gallons)	In Town Rates		Usage (Gallons)	In Town Rates	
	New Rate	Old Rate		New Rate	Old Rate
0	\$ 31.45	\$ 26.93	0	\$ 35.00	\$ 26.93
3,000	\$ 45.79	\$ 40.40	3,000	\$ 49.04	\$ 40.40
4,700	\$ 53.92	\$ 48.03	4,700	\$ 57.00	\$ 48.03
10,000	\$ 79.25	\$ 71.83	10,000	\$ 81.80	\$ 71.83
35,500	\$ 225.74	\$ 209.29	35,500	\$ 225.74	\$ 209.29

### WATER RATE ANALYSIS SUMMARY

**2006 Average Rate** = \$36.85/ERU/Month

**Est. Required FY 2008 Avg. Rate** = \$40.52/ERU/Month  
(including tax revenue)

#### Recommendations

1. Continue to review annually the ERU value for commercial connections.
2. Water rates and fees should be reviewed by the Town Council periodically to ensure that they remain abreast of actual inflation rates and costs.

## SECTION X IMPACT FEES

### A. IMPACT FEE

It is recommended that an impact fee should be charged to all new connections to the culinary water system. An impact fee that is charged by a community may be used to pay for the debt service associated with surplus capacity built into the system. The surplus capacity in the water system has been designed for growth, and for this reason, impact fees should pay for that portion of the debt service associated with the system surplus capacity. The impact fee should also be used to pay for the cost of improvements to the system that are required to support new growth as new connections are added to the system. The existing impact fees can be found in Table X.A.1 below.

**Table X.A.1 Existing Impact Fee**

Connection	Impact Fee
3/4"	\$3,725
1"	\$6,625
1 1/2"	\$14,910
2"	\$23,505
3"	\$59,635
4"	\$106,020
6"	\$238,550

### B. CALCULATION

The total cost that is eligible for impact fee calculation is equal to the existing debt service from previous water improvements projects that can be attributed to new growth plus the portion of the proposed water improvements project that will be constructed to accommodate new growth. The combined total cost that is due to new growth is divided by the number of new ERUs that will be added to the system during the 20-year planning period.

The impact fee calculation found in Table

X.B.1 shows that the maximum impact fee that the Town of Springdale may assess each new ERU is \$6,060 which includes existing debt service previously included in the former impact fee calculations. The Town is free to charge less than the maximum if it decides to do so, but should ensure that collected impact fees be sufficient to cover future culinary water system needs and payments made for debt incurred for this project. Please note that this impact fee calculation assumes that the system improvements constructed, the expenses for those improvements and funding package match those in this report. Should any of these conditions change, the impact fee should be adjusted accordingly.

The percentage eligible for impact fees for the new debt service was determined by whether the improvements were needed to allow the system to meet State requirements now or in the future. The percent of impact fee eligible costs for the recommended project in this plan average 85%.

When the Town allows connections larger than the assumed  $\frac{3}{4}$ " residential connection, larger impact fees should be charged for these larger connections as has been done in the past. Table X.B.1 includes at the bottom the proposed impact fees associated with the size of the connection required and assumes that a  $\frac{3}{4}$ " water service connection is equivalent to 1 ERU. Each ERU with a  $\frac{3}{4}$ " connection would then pay an impact fee of \$6,060 or the amount the Town of Springdale decides to charge. For larger meters the base impact fee of \$6,060 is increased according to the percent increase in flow capacity of each size of meter.

TABLE X.B.1

**TOWN OF SPRINGDALE**  
**IMPACT FEE ANALYSIS FY 2007**  
**CULINARY WATER MASTER PLAN**

**EXISTING DEBT SERVICE**

	<b>% Eligible</b>	<b>Eligible</b>
No Interest State Loan	100%	\$ 102,480
Water Revenue Bond 1995A	100%	\$ 306,000
Water Revenue Bond 2004 Tank Project	100%	\$ 1,014,890
Existing Impact Fee Eligible Cost:		\$ 1,423,370

**PROPOSED IMPROVEMENT PROJECTS**

Total Estimated 2008 Project Cost	\$ 2,968,800
Total Estimated 2008 Project Cost Paid in Grant Funds	\$ (768,800)
% Of New Project Cost Due to New Growth	85.2%
Interest From New Debt Service	\$324,887
Impact Fee Eligible Cost	<u>\$ 276,889</u>

No. of ERUs (2007)	699
Future ERUs (2028 Historic)	1,289
No. of New ERUs Due to Growth	<u>590</u>

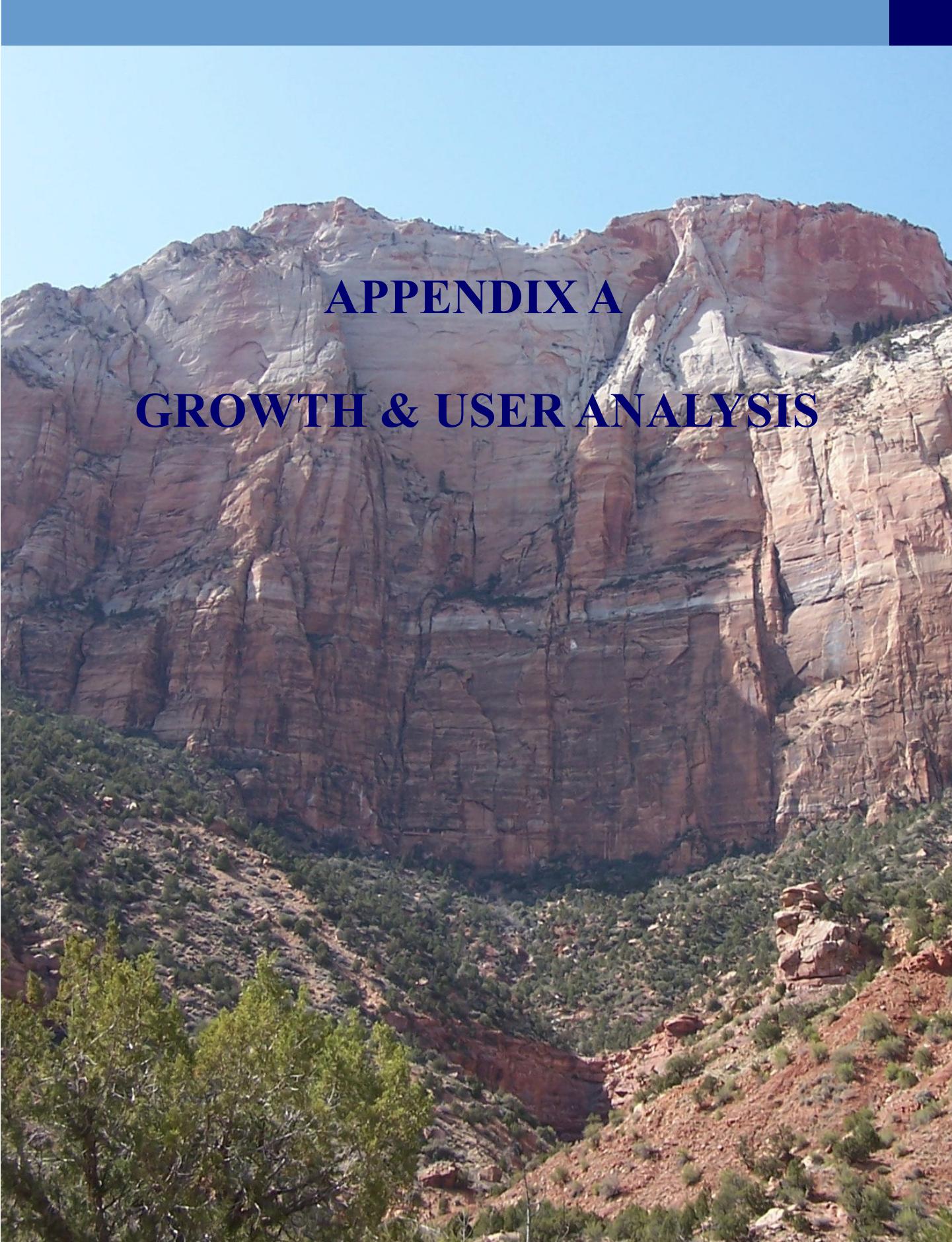
Impact Fee Eligible Cost for Improvement Projects	\$ 3,575,239
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Impact Fee Amount for Improvement Projects = Total Eligible Cost / New ERUs      \$ 6,060 /Conn.

Additional Amount per connection for Future Water Rights      \$ - /Conn.

Proposed Impact Fee for Town of Springdale Water Users (FY2007) =      \$ 6,060 /Conn.

Meter Size	Cross-Sectional Area (in <sup>2</sup> )	% Area Increase	Impact Fee
3/4"	0.44	0%	\$ 6,060
1"	0.79	80%	\$ 10,880
1 1/2"	1.77	302%	\$ 23,377
2"	3.14	614%	\$ 43,244
3"	7.07	1507%	\$ 97,369
4"	12.57	2757%	\$ 173,115
6"	28.27	6325%	\$ 389,338



# APPENDIX A

## GROWTH & USER ANALYSIS

## Water Usage Data

Month & Year	Water Usage Data				Connection Data		
	Residential	Commercial	Other	Total	Residential	Commercial	Other
Jan-05	598	1,146	4	1,748	206	91	4
Feb-05	590	1,015	4	1,609	208	91	4
Mar-05	774	2,343	6	3,122	208	91	4
Apr-05	766	2,623	31	3,420	207	91	4
May-05	1,099	3,892	13	5,004	213	91	4
Jun-05	1,109	4,065	30	5,204	211	92	4
Jul-05	1,117	3,886	12	5,015	211	92	4
Aug-05	1,232	4,868	14	6,114	215	95	4
Sep-05	1,365	4,193	6	5,564	211	94	3
Oct-05	2,822	13,492	13	16,328	211	94	3
Nov-05	974	7,746	9	8,729	211	94	4
Dec-05	742	1,425	3	2,170	212	94	4
Jan-06	748	1,553	5	2,306	214	96	4
Feb-06	758	1,391	6	2,155	216	94	4
Mar-06	831	1,973	6	2,809	217	95	4
Apr-06	703	2,634	9	3,345	215	95	4
May-06	1,223	3,505	13	4,741	214	94	4
Jun-06	1,354	4,901	10	6,264	218	94	4
Jul-06	951	3,234	11	4,196	215	94	4
Aug-06	1,289	4,557	16	5,862	217	94	4
Sep-06	1,141	3,704	13	4,857	217	96	4
Oct-06	886	3,429	20	4,335	221	95	4
Nov-06	784	2,017	26	2,828	218	97	4
Dec-06	884	1,527	35	2,446	219	95	5
Jan-07	865	1,142	26	2,033	219	95	5
Feb-07	700	1,360	29	2,088	221	96	6
Mar-07	791	2,308	38	3,137	221	96	5
Apr-07	879	3,344	39	4,262	221	95	5
May-07	1,108	4,073	35	5,215	221	95	5
Jun-07	1,214	4,353	14	5,580	220	95	5
Jul-07	1,792	5,250	12	7,054	222	95	5
Aug-07	1,189	4,203	9	5,401	224	95	5
Sep-07	1,090	3,715	8	4,812	222	95	6
Oct-07	1,089	3,950	23	5,062	223	95	6
Nov-07	820	2,200	22	3,042	224	95	6
Dec-07	673	1,167	3	1,842	222	96	6
Jan-08	713	1,534	4	2,251	223	96	6
Feb-08	609	1,311	8	1,928	228	96	6
Mar-08	809	2,692	7	3,508	223	95	6
<b>Total</b>	<b>27,608</b>	<b>127,718</b>	<b>590</b>	<b>167,386</b>			

Year	Yearly Usage in 1,000 Gallons				Average Yearly Connections		
2005	13,186	50,695	145	64,026	210	93	4
2006	11,552	34,423	169	46,144	217	95	4
2007	12,209	37,063	257	49,530	222	95	5
2008	2,131	5,537	19	7,687	225	96	6

Year	Monthly Averages by Year				Weighted Usage (Winter)		
2005 Ave	1,098,833	4,224,583	12,083	5,335,500	62	255	20
2006 Ave	962,667	2,868,583	14,083	3,845,333	114	504	40
2007 Ave	1,017,417	3,088,583	21,417	4,127,500	121	465	196
2008 Ave	1,065,500	2,768,500	9,500	3,843,500	107	616	26

Year	Average Usage Per Connection				
	Residential		Commercial		
	Monthly	Daily	Monthly	Daily	
2005	5,233	172	45,426	1,493	
2006	4,436	146	30,196	993	
2007	4,583	151	32,511	1,069	
2008	4,736	156	28,839	948	

Table II. C-1

Year	Residential Connections	Commercial ERUs	Other ERUs	Total ERU's	Res. Conn.	Rate
					2005	210
2005	210	93	4	307	2005	210
2006	217	95	4	316	2006	217
2007	222	95	5	322	2007	222
*2008	225	96	6	327	2008	225
*2009	232	99	0	331	2009	232

\* Future connections projected using assumed growth rate

ASSUMED GROWTH RATE **3.10%**

## Average Usage Per Connection

	2005	2006	2007	2008	Average
<b>Residential</b>					<b>05 - 07</b>
Usage (gallons)	13,186,000	11,552,000	12,209,000	2,131,000	12,315,667
Connections (ERUs)	210	217	222	225	216
Usage Per Connection (gal/year)	62,790	53,235	54,995	9,471	57,007
Daily Usage Per Connection (gal/day)	172	146	151	158	<b>156</b>
This master plan will use a historical daily ERU usage of					250 gpd/conn.
This master plan will also use the state standard daily ERU usage of					400 gpd/conn.
<b>Commercial</b>					
Usage (gallons)	50,695,000	34,423,000	37,063,000	5,537,000	40,727,000
Connections	93	95	95	96	94
Usage Per Connection (gal/year)	545,108	362,347	390,137	57,677	432,531
Daily Usage Per Connection (gal/day)	1,493	993	1,069	961	1,185
Equivalent Residential Unit - Historical	5.97	3.97	4.28	3.85	4.74
Equivalent Residential Unit - State	3.73	2.48	2.67	2.40	2.96
Commercial ERUs - Historical	556	377	406	369	406
Commercial ERUs - State	347	236	254	231	254
<b>Other</b>					
Usage (gallons)	145,000	169,000	257,000	19,000	190,333
Connections	4	4	5	6	4
Usage Per Connection (gal/year)	36,250	42,250	51,400	3,167	43,300
Daily Usage Per Connection (gal/day)	99	116	141	9	119
Equivalent Residential Unit	0.58	0.79	0.93	0.33	1
Other ERUs	4	4	5	6	5
Total ERUs	770	598	633	600	<b>667</b>

\*2008 numbers are shown but only for the first 3 months of the year. 2008 data is not used in the average calculations shown.

gallons per connection per day will be used in this master plan for current usage.

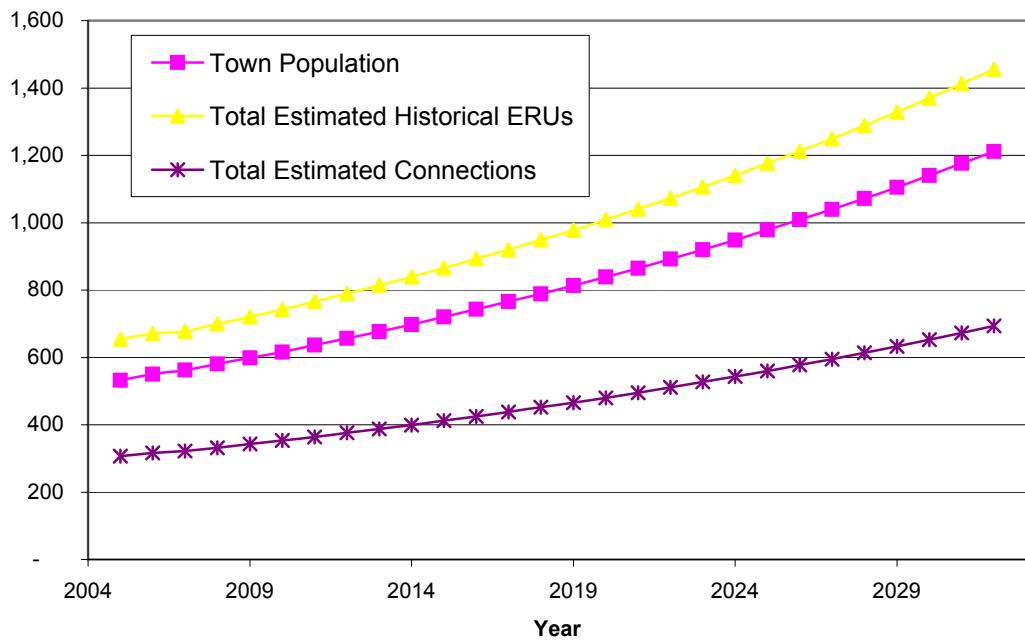
Average Usage (Winter Versus Summer)	Residential	Commercial	Other
Yearly Average Usage	156	1,185	gpd/conn
Indoor Usage (December,January,February Ave Month Usage)	101	460	gpd/conn
Outdoor Usage (Total-Indoor)	55	725	gpd/conn

\*For Residential Connections the average winter use can be considered indoor use in the summer with the remainder being outdoor use the rest of the year. For commercial connections this increase is likely due to tourist season being during the summer months and not outdoor use.

## Population Data

	Population	% Growth
1970 Census Population	182	
1980 Census Population	258	3.6%
1990 Census Population	275	0.6%
2000 Census Population	457	5.2%
2006 Estimated Population	550	3.1%
3.6% Growth rate experienced between 1970 & 1980		
0.6% Growth rate experienced between 1980 & 1990		
2.1% Growth rate experienced between 1970 & 1990		
5.2% Growth rate experienced between 1990 & 2000		
3.1% Growth rate experienced between 1970 & 2000		
3.1% Growth rate experienced between 2000 & 2006		
3.1% Growth rate experienced between 1970 & 2006		
Growth Rate for 20 Year Residential Projections		3.1%
Growth Rate for 20 Year Commercial Projections		3.1%
Projected 20 Year Population (2027)	1,039	
Projected 20 Year ERU's (2027)	1,249	

## Springdale Culinary Water Master Plan Projected Growth



Year	Est. Residential Growth Rate	Total Estimated Connections	Total Estimated Historical ERUs	Total Estimated State ERUs	Estimated Population
2005	-	307	654	489	532
2006	-	316	671	502	550
2007	-	322	677	508	563
2008	3.1%	332	699	525	580
2009	3.1%	343	720	541	598
2010	3.1%	353	742	558	616
2011	3.1%	364	766	575	636
2012	3.1%	376	790	593	656
2013	3.1%	388	814	612	677
2014	3.1%	399	839	630	697
2015	3.1%	412	866	650	720
2016	3.1%	425	893	671	743
2017	3.1%	438	920	691	765
2018	3.1%	452	949	713	788
2019	3.1%	466	978	735	814
2020	3.1%	480	1,009	758	839
2021	3.1%	495	1,040	781	864
2022	3.1%	511	1,073	806	892
2023	3.1%	527	1,106	831	920
2024	3.1%	543	1,140	856	948
2025	3.1%	560	1,176	883	978
2026	3.1%	578	1,212	911	1,009
2027	3.1%	595	1,249	939	1,039
2028	3.1%	614	1,288	968	1,072
2029	3.1%	633	1,328	998	1,105
2030	3.1%	653	1,370	1,029	1,141
2031	3.1%	673	1,413	1,061	1,176
2032	3.1%	694	1,456	1,094	1,212

\* 2006 Data is based on an estimate provided by the US Census.

\*\* Estimated Population is determined by multiplying the Estimated Residential ERU's by 2.46.

## Buildout Calculations

From Town of Springdale

Town of Springdale Buildout Estimates			ERC	Current	Estimated
	Current	Estimated		ERUs	ERUs
Residential Connections	222	1022	1	222	1022
Hotel Rooms	666	1666	0.5	333	833
Restaurants	12	30	5.5	66	165
Other connections ERU	24	50	2	48	100
Build Out	924	2768		669	2120

From Springdale General Plan Dec. 2005 - Simple Acreage Division

Zones	Acres	Acres Vacant	Density	Existing	Potential	Total Buildout
Residential	1201	721		206		616
Foothill Residential	1053	661	2		330	
Valley Residential	148	60	0.75		80	
Commercial	269	114		91		381
Central Commercial	76	32	0.25		128	
Village Commercial	193	81	0.5		162	
Public Use	244	31	5	6	6	12
Agriculture	19	0	5	1	0	1
						1010



# APPENDIX B

## FIVE POINT ANALYSIS

## Culinary Water Right Data

Culinary Water Rights		Flow		
W.R. #	Source	gpm	cfs	AcFt.
81-105	Spring above ZNP Campground	7.2	0.016	11.58
81-220	Birch Springs East - West of ZNP Museum	18.8	0.042	30.41
81-274	Birch Springs West - West of ZNP Museum	31.4	0.070	50.68
81-585	Hummingbird Well	148.1	0.330	238.91
81-1326	Cemetery Well	65.1	0.145	104.98
81-2413	Big Springs	235.6	0.525	380.08
81-3392	Springdale Town for Municipal Use - Irrigation	596.9	1.330	365.95
<b>Total Water Rights</b>		<b>1,103.2</b>	<b>2.458</b>	<b>1,182.6</b>
<b>Other Water Rights of Interest (Slated for Irrigation)</b>		<b>Flow</b>		
W.R. #	Source/Owner	gpm	cfs	AcFt.
81-1142	North Fork Virgin River	659.7	1.47	1,064.2
	Springdale Consolidated Irrigation Company			
	Total Other Water Rights	660	1.47	1,064.2

### B. Current & Projected Required Water Right (2008-2028+):

Average Water Right Required (Historic Usage)		2008	2028	Buildout	
Residential ERUs		229	423	1,022	ERUs
Commercial ERUs (Historic)		464	855	998	ERUs
Other ERUs		6	11	100	ERUs
Total ERUs		699	1,289	2,120	ERU's
Average Residential Water Use (Indoor + Outdoor) (gpd)		250	250	250	gpd
Average Commercial Water Use (Indoor + Outdoor) (gpd)		250	250	250	gpd/ERU
Average Other Water Use (Indoor + Outdoor) (gpd)		250	250	250	gpd/ERU
Required Water Right for Residential Use (Indoor + Outdoor) (gpm)		40	73	177	gpm
Required Water Right for Residential Use (Indoor + Outdoor) (AcFt)		64	118	286	Ac-Ft
Required Water Right for Commercial Use (gpm)		81	148	173	gpm
Required Water Right for Commercial Use (AcFt)		130	239	279	Ac-Ft
Required Water Right for Other Use (gpm)		1	2	17	gpm
Required Water Right for Other Use (AcFt)		2	3	28	Ac-Ft
Culinary System Water Right Surplus/(Deficit) (gpm)		982	879	735	gpm
Culinary System Water Right Surplus/(Deficit) (AcFt)		987	822	589	Ac-Ft
Average Water Right Required (State Usage)		2008	2028	Buildout	
Residential ERUs		229	423	1,022	ERUs
Commercial ERUs (State)		290	534	998	ERUs
Other ERUs		6	11	100	ERUs
Total ERUs		525	968	2,120	ERU's
Average Residential Water Use (Indoor + Outdoor) (gpd)		400	400	400	gpd
Average Commercial Water Use (Indoor + Outdoor) (gpd)		400	400	400	gpd/ERU
Average Other Water Use (Indoor + Outdoor) (gpd)		400	400	400	gpd/ERU
Required Water Right for Residential Use (Indoor + Outdoor) (gpm)		64	118	284	gpm
Required Water Right for Residential Use (Indoor + Outdoor) (AcFt)		103	190	458	Ac-Ft
Required Water Right for Commercial Use (gpm)		81	148	277	gpm
Required Water Right for Commercial Use (AcFt)		130	239	447	Ac-Ft
Required Water Right for Other Use (gpm)		2	3	28	gpm
Required Water Right for Other Use (AcFt)		3	5	45	Ac-Ft
Culinary System Water Right Surplus/(Deficit) (gpm)		957	834	514	gpm
Culinary System Water Right Surplus/(Deficit) (AcFt)		947	749	233	Ac-Ft

**B. Current Required Water Right**

Using Town of Springdale Historic Average Consumption

250 gpd/conn.

Table 3.B-1

<b>Average Demand (Total Use)</b>					
<b>Residential Use</b>					
229 ERUs X	<u>250 gpd X</u>	<u>1 day X</u>	<u>1 hr</u>	=	40 gpm
	ERU	24 hr	60 min.		
<b>Commercial Use</b>					
464 ERUs X	<u>250 gpd X</u>	<u>365 day X</u>	<u>1 Acft.</u>	=	81 gpm
	ERU	1 yr	325,829 gal		
<b>Other Use</b>					
6 ERUs X	<u>250 gpd X</u>	<u>1 day X</u>	<u>1 hr</u>	=	1 gpm
	ERU	24 hr	60 min.		
6 ERUs X	<u>250 gpd X</u>	<u>365 day X</u>	<u>1 Acft.</u>	=	2 acft
	ERU	1 yr	325,829 gal		
<b>Total Required Water Right</b>				<b>196 Acft</b>	<b>121 gpm</b>
<b>Existing Culinary System Water Right Surplus</b>				<b>987 Acft</b>	<b>982 gpm</b>

**Projected 20 Year Required Water Right**

Using Town of Springdale Historic Average Consumption

250 gpd/conn.

c. Table 3.C-1

<b>Average Demand (Total Use)</b>					
<b>Residential Use</b>					
423 ERU's X	<u>250 gpd X</u>	<u>1 day X</u>	<u>1 hr</u>	=	73 gpm
	ERU	24 hr	60 min.		
<b>Commercial Use</b>					
855 ERU's X	<u>250 gpd X</u>	<u>365 day X</u>	<u>1 Acft.</u>	=	148 gpm
	ERU	1 yr	325,829 gal		
<b>Other Use</b>					
11 ERU's X	<u>250 gpd X</u>	<u>1 day X</u>	<u>1 hr</u>	=	2 gpm
	ERU	24 hr	60 min.		
11 ERU's X	<u>250 gpd X</u>	<u>365 day X</u>	<u>1 Acft.</u>	=	3 acft
	ERU	1 yr	325,829 gal		
<b>Total Required Water Right</b>				<b>361 Acft</b>	<b>224 gpm</b>
<b>Projected Culinary System Water Right Surplus</b>				<b>822 Acft</b>	<b>879 gpm</b>

**Projected Buildout Required Water Right**

Using Town of Springdale Historic Average Consumption

250 gpd/conn.

D. Table 3.D-1

<b>Average Demand (Total Use)</b>				
Residential Use				
1,022 ERU's X	<u>250 gpd X</u>	<u>1 day X</u>	<u>1 hr</u>	=
	ERU	24 hr	60 min.	177 gpm
Commercial Use				
1,022 ERU's X	<u>250 gpd X</u>	<u>365 day X</u>	<u>1 Acft.</u>	=
	ERU	1 yr	325,829 gal	286 acft
Other Use				
998 ERU's X	<u>250 gpd X</u>	<u>1 day X</u>	<u>1 hr</u>	=
	ERU	24 hr	60 min.	173 gpm
998 ERU's X	<u>250 gpd X</u>	<u>365 day X</u>	<u>1 Acft.</u>	=
	ERU	1 yr	325,829 gal	279 acft
<b>Total Required Water Right</b>				
			<b>594 Acft</b>	<b>368 gpm</b>
<b>Projected Culinary System Water Right Surplus</b>				
			<b>589 Acft</b>	<b>735 gpm</b>

**E. Current Required Water Right**

Using Town of Springdale State Standard Average Consumption

400 gpd/conn.

Table 3.E-1

<b>Average Demand (Total Use)</b>				
Residential Use				
229 ERUs X	<u>400 gpd X</u>	<u>1 day X</u>	<u>1 hr</u>	=
	ERU	24 hr	60 min.	64 gpm
Commercial Use				
229 ERUs X	<u>400 gpd X</u>	<u>365 day X</u>	<u>1 Acft.</u>	=
	ERU	1 yr	325,829 gal	103 acft
Other Use				
290 ERUs X	<u>400 gpd X</u>	<u>1 day X</u>	<u>1 hr</u>	=
	ERU	24 hr	60 min.	81 gpm
290 ERUs X	<u>400 gpd X</u>	<u>365 day X</u>	<u>1 Acft.</u>	=
	ERU	1 yr	325,829 gal	130 acft
<b>Total Required Water Right</b>				
			<b>235 Acft</b>	<b>146 gpm</b>
<b>Existing Culinary System Water Right Surplus</b>				
			<b>947 Acft</b>	<b>957 gpm</b>

### Projected 20 Year Required Water Right

Using Town of Springdale State Standard Average Consumption

400 gpd/conn.

F. Table 3.F-1

<b>Average Demand (Total Use)</b>					
<b>Residential Use</b>					
423 ERU's X	<u>400 gpd X</u>	1 day X	1 hr	=	118 gpm
	ERU	24 hr	60 min.		
<b>Commercial Use</b>					
534 ERU's X	<u>400 gpd X</u>	365 day X	1 Acft.	=	148 gpm
	ERU	1 yr	325,829 gal		
<b>Other Use</b>					
11 ERU's X	<u>400 gpd X</u>	1 day X	1 hr	=	3 gpm
	ERU	24 hr	60 min.		
11 ERU's X	<u>400 gpd X</u>	365 day X	1 Acft.	=	5 acft
	ERU	1 yr	325,829 gal		
<b>Total Required Water Right</b>				<b>434 Acft</b>	<b>269 gpm</b>
<b>Projected Culinary System Water Right Surplus</b>				<b>749 Acft</b>	<b>834 gpm</b>

### Projected Buildout Required Water Right

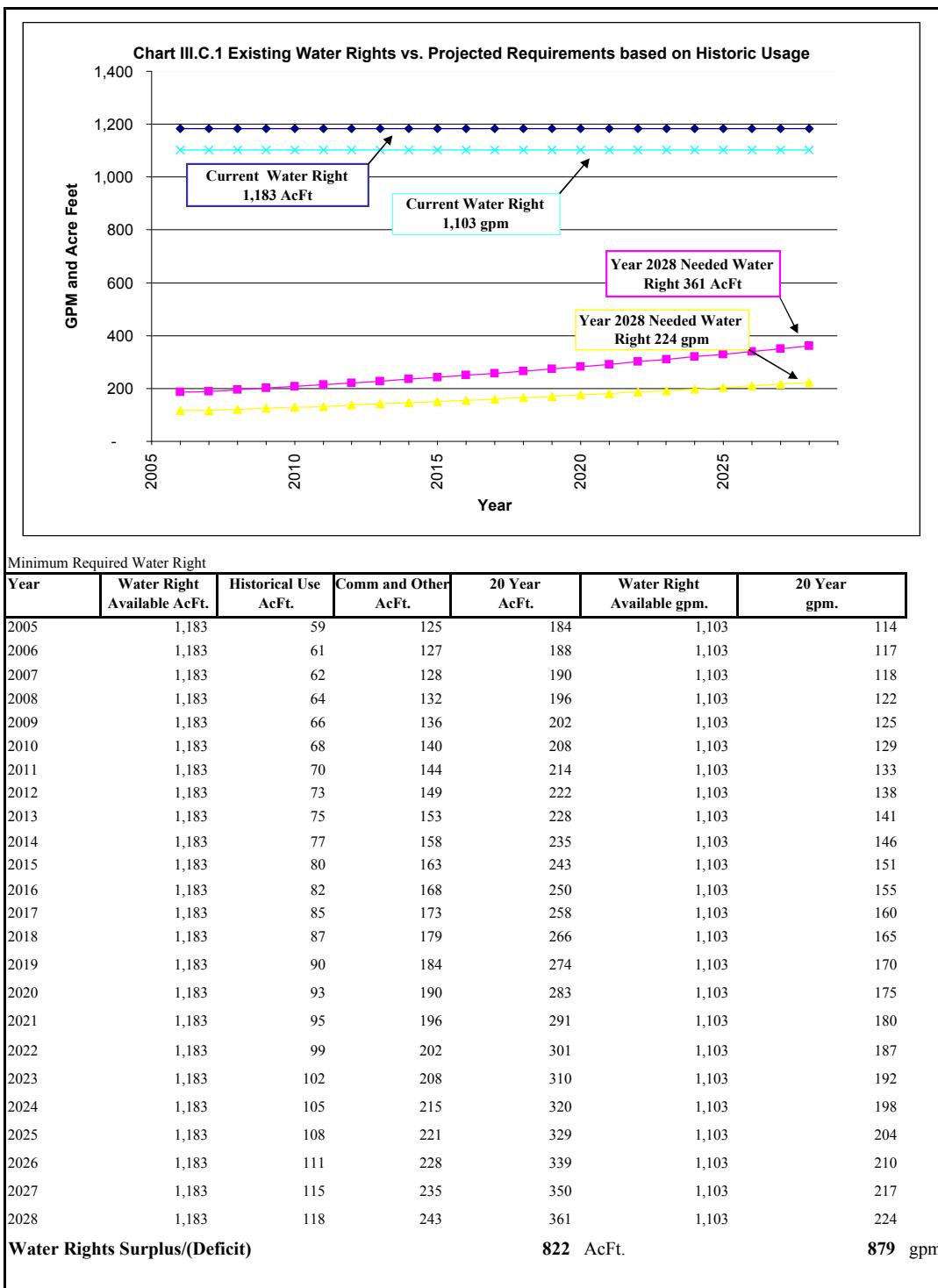
Using Town of Springdale State Standard Average Consumption

400 gpd/conn.

G. Table 3.G-1

<b>Average Demand (Total Use)</b>					
<b>Residential Use</b>					
1,022 ERU's X	<u>400 gpd X</u>	1 day X	1 hr	=	284 gpm
	ERU	24 hr	60 min.		
<b>Commercial Use</b>					
998 ERU's X	<u>400 gpd X</u>	365 day X	1 Acft.	=	458 acft
	ERU	1 yr	325,829 gal		
<b>Other Use</b>					
100 ERU's X	<u>400 gpd X</u>	1 day X	1 hr	=	28 gpm
	ERU	24 hr	60 min.		
100 ERU's X	<u>400 gpd X</u>	365 day X	1 Acft.	=	45 acft
	ERU	1 yr	325,829 gal		
<b>Total Required Water Right</b>				<b>950 Acft</b>	<b>589 gpm</b>
<b>Projected Culinary System Water Right Surplus</b>				<b>233 Acft</b>	<b>514 gpm</b>

<b>Water Rights Summary (Not Including Irrigation)</b>				
	<b>Historic</b>		<b>State</b>	
Existing Water Rights	<b>1,182.6</b>	Acft	<b>1,182.6</b>	Acft
Existing Water Rights Surplus	<b>986.8</b>	Acft	<b>947.4</b>	Acft
Projected 2028 Water Rights Surplus	<b>821.6</b>	Acft	<b>748.9</b>	Acft
Projected Buildout Water Rights Surplus	<b>588.9</b>	Acft	<b>232.6</b>	Acft
Surplus ERUs Serviceable at Buildout	<b>2,102</b>		<b>519</b>	



## Water Source Capacity

A. Town of Springdale Sources	Total Flow	
	CFS	gpm
Spring above ZNP Campground	0.000	0
Birch Springs East - West of ZNP Museum	0.000	0
Birch Springs West - West of ZNP Museum	0.000	0
Hummingbird Well	0.000	0
Cemetery Well	0.000	0
Big Springs	0.000	0
North Fork of the Virgin River	0.891	400
<b>Source Total =</b>	<b>0.891</b>	<b>400</b>

### B. Current & Projected Required Water Source (2008-2028+):

Average Source Required - Historic Use	Year	2008	2028	Buildout
Residential ERUs	229	423	1,022	ERUs
Commercial ERUs	464	855	998	
Other ERUs	6	11	100	
Total ERUs	699	1,289	2,120	
Residential/Other Peak Day Average Water Use (2 X Ave. Day)	500	500	500	
Commercial Peak Day Average Water Use (2 X Ave. Day)	500	500	500	
Required Water Source for Residential Use	80	147	355	
Required Water Source for Commercial Use	161	297	347	
Required Water Source for Other Use	2	4	35	
Total Required Water Source	243	448	736	
Culinary System Water Source Surplus/(Deficit)	157	(48)	(336)	gpm

### C. Current & Projected Required Water Source (2008-2028):

Average Source Required - State Std. Use	Year	2008	2028	Buildout
Residential ERUs	229	423	1,022	ERUs
Commercial ERUs	290	534	998	
Other ERUs	6	11	100	
Total ERUs	525	968	2,120	
Residential/Other Peak Day Average Water Use (2 X Ave. Day)	800	800	800	
Commercial Peak Day Average Water Use (2 X Ave. Day)	800	800	800	
Required Water Source for Residential Use	127	235	568	
Required Water Source for Commercial Use	161	297	554	
Required Water Source for Other Use	3	6	56	
Total Required Water Source	292	538	1,178	
Culinary System Water Source Surplus/(Deficit)	108	(138)	(778)	gpm

Historic Usage Source

Required Indoor/Outdoor Source - Historic Usage					
Residential ERUs					
229 ERUs X	500 gpd X	1 day X	1 hr	=	80 gpm
	ERU	24 hr	60 min.		
Commercial ERUs					
464 ERUs X	500 gpd X	1 day X	1 hr	=	161 gpm
	ERU	24 hr	60 min.		
Other ERUs					
6 ERUs X	500 gpd X	1 day X	1 hr	=	2 gpm
	ERU	24 hr	60 min.		
Outdoor Use					
- Irrigated Acres X	4.9 gpm			=	0 gpm
		irrigated acre in zone 6			
Total Required Source Capacity					
					243 gpm
Existing Culinary System Source Capacity Surplus					
					157 gpm

State Standard Source

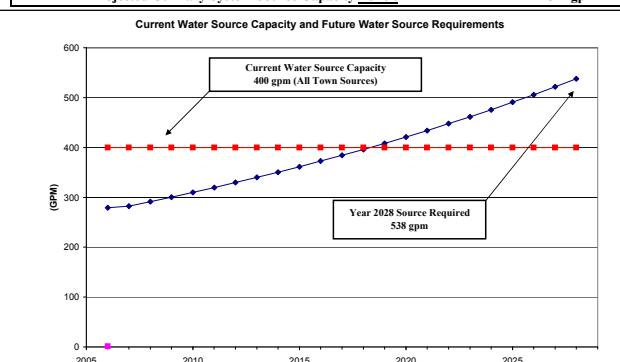
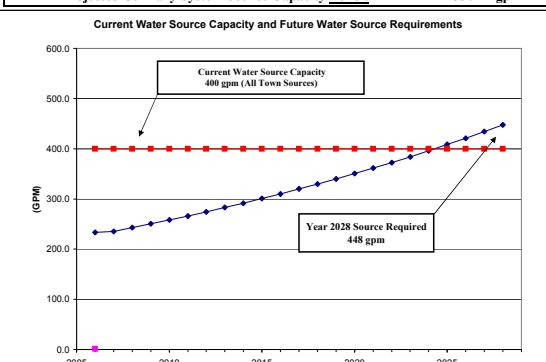
Required Indoor/Outdoor Source - Historic Usage					
Residential ERUs					
229 ERUs X	800 gpd X	1 day X	1 hr	=	127 gpm
	ERU	24 hr	60 min.		
Commercial ERUs					
290 ERUs X	800 gpd X	1 day X	1 hr	=	161 gpm
	ERU	24 hr	60 min.		
Other ERUs					
6 ERUs X	800 gpd X	1 day X	1 hr	=	3 gpm
	ERU	24 hr	60 min.		
Outdoor Use					
- Irrigated Acres X	4.9 gpm			=	0 gpm
		irrigated acre in zone 6			
Total Required Source Capacity					
					292 gpm
Existing Culinary System Source Capacity Surplus					
					108 gpm

Required Indoor/Outdoor Source - Historic Usage					
Residential ERUs					
423 ERUs X	500 gpd X	1 day X	1 hr	=	147 gpm
	ERU	24 hr	60 min.		
Commercial ERUs					
855 ERUs X	500 gpd X	1 day X	1 hr	=	297 gpm
	ERU	24 hr	60 min.		
Other ERUs					
11 ERUs X	500 gpd X	1 day X	1 hr	=	4 gpm
	ERU	24 hr	60 min.		
Outdoor Use					
- Irrigated Acres X	4.9 gpm			=	0 gpm
		irrigated acre in zone 6			
Total Required Source Capacity					
					448 gpm
Projected Culinary System Source Capacity Deficit					
					-48 gpm

Required Indoor/Outdoor Source - Historic Usage					
Residential ERUs					
423 ERUs X	800 gpd X	1 day X	1 hr	=	235 gpm
	ERU	24 hr	60 min.		
Commercial ERUs					
534 ERUs X	800 gpd X	1 day X	1 hr	=	297 gpm
	ERU	24 hr	60 min.		
Other ERUs					
11 ERUs X	800 gpd X	1 day X	1 hr	=	6 gpm
	ERU	24 hr	60 min.		
Outdoor Use					
- Irrigated Acres X	4.9 gpm			=	0 gpm
		irrigated acre in zone 6			
Total Required Source Capacity					
					538 gpm
Projected Culinary System Source Capacity Deficit					
					-138 gpm

Required Indoor/Outdoor Source - Historic Usage					
Residential ERUs					
1,022 ERUs X	500 gpd X	1 day X	1 hr	=	355 gpm
	ERU	24 hr	60 min.		
Commercial ERUs					
998 ERUs X	500 gpd X	1 day X	1 hr	=	347 gpm
	ERU	24 hr	60 min.		
Other ERUs					
100 ERUs X	500 gpd X	1 day X	1 hr	=	35 gpm
	ERU	24 hr	60 min.		
Outdoor Use					
- Irrigated Acres X	4.9 gpm			=	0 gpm
		irrigated acre in zone 6			
Total Required Source Capacity					
					736 gpm
Projected Culinary System Source Capacity Deficit					
					-336 gpm

Required Indoor/Outdoor Source - Historic Usage					
Residential ERUs					
1,022 ERUs X	800 gpd X	1 day X	1 hr	=	568 gpm
	ERU	24 hr	60 min.		
Commercial ERUs					
998 ERUs X	800 gpd X	1 day X	1 hr	=	554 gpm
	ERU	24 hr	60 min.		
Other ERUs					
100 ERUs X	800 gpd X	1 day X	1 hr	=	56 gpm
	ERU	24 hr	60 min.		
Outdoor Use					
- Irrigated Acres X	4.9 gpm			=	0 gpm
		irrigated acre in zone 6			
Total Required Source Capacity					
					1,178 gpm
Projected Culinary System Source Capacity Deficit					
					-778 gpm



Water Source Capacity Summary - Historic Usage					
Existing Water Source		400	GPM		
Existing Water Source Surplus		157	GPM		
Projected 2028 Water Source Surplus		-48	GPM		
Projected Buildout Water Source Deficit		-336	GPM		
ERUs Serviceable at Buildout with Current Water Source		(968)			

Water Source Capacity Summary - State Standard Usage					
Existing Water Source		400	GPM		
Existing Water Source Surplus		108	GPM		
Projected 2028 Water Source Surplus		-138	GPM		
Projected Buildout Water Source Deficit		-778	GPM		
ERUs Serviceable at Buildout with Current Water Source		(1,400)			

## Water Storage Capacity

### A. Existing Storage Capacity:

North Concrete Tank	500,000	gal.
East Concrete Tank	-	gal.
Anasazi Steel Tank	200,000	gal.
<b>Total Existing Storage Capacity</b>	<b>700,000</b>	<b>gal.</b>

## Historic Usage

### B. Existing Required Storage Capacity

Residential ERUs

$$\frac{250 \text{ gpd}}{\text{ERU}} \times 229 \text{ ERUs} = 57,250 \text{ gpd}$$

Commercial ERUs

$$\frac{250 \text{ gpd}}{\text{ERU}} \times 464 \text{ ERUs} = 116,065 \text{ gpd}$$

Other ERUs

$$\frac{250 \text{ gpd}}{\text{ERU}} \times 6 \text{ ERUs} = 1,500 \text{ gpd}$$

Fire Demand

$$\frac{3,500 \text{ gpm}}{1 \text{ hr}} \times 60 \text{ min} \times 3 \text{ hr} = 630,000 \text{ gal.}$$

Emergency Supply

$$25\% \text{ of required storage} = 201,204 \text{ gal.}$$

$$\text{Total Existing Required Storage} = 1,006,019 \text{ gal.}$$

$$\text{Total Existing Storage Capacity} = 700,000 \text{ gal.}$$

$$\text{Existing Storage Capacity Deficit} = \text{Total Existing Required Storage} - \text{Total Existing Storage Capacity} = 306,019 \text{ gal.}$$

### Projected Required Storage Capacity in 2028

Residential ERUs

$$\frac{250 \text{ gpd}}{\text{ERU}} \times 423 \text{ ERUs} = 105,750 \text{ gpd}$$

Commercial ERUs

$$\frac{250 \text{ gpd}}{\text{ERU}} \times 855 \text{ ERUs} = 213,734 \text{ gpd}$$

Other ERUs

$$\frac{250 \text{ gpd}}{\text{ERU}} \times 11 \text{ ERUs} = 2,762 \text{ gpd}$$

Fire Flow

$$\frac{3,500 \text{ gpm}}{1 \text{ hr}} \times 60 \text{ min} \times 3 \text{ hr} = 630,000 \text{ gal.}$$

Emergency Supply

$$25\% \text{ of required storage} = 238,062 \text{ gal.}$$

$$\text{Total Required Storage} = 1,190,308 \text{ gal.}$$

$$\text{Total Existing Storage Capacity} = 700,000 \text{ gal.}$$

$$\text{Future Storage Capacity Deficit} = \text{Total Required Storage} - \text{Total Existing Storage Capacity} = 490,308 \text{ gal.}$$

## State Standard Usage

**Projected Required Storage Capacity in 2028**

Residential ERUs

400 gpd	X	423 ERUs	=	169,200 gpd
ERU				

Commercial ERUs

400 gpd	X	534 ERUs	=	213,554 gpd
ERU				

Other ERUs

400 gpd	X	11 ERUs	=	4,420 gpd
ERU				

Fire Flow

3,500 gpm	X	60 min	X	3 hr =	630,000 gal.
		1 hr			

Emergency Supply

25% of required storage		254,293 gal.
-------------------------	--	--------------

<b>Total Required Storage</b>	1,271,467 gal.
-------------------------------	----------------

<b>Total Existing Storage Capacity</b>	700,000 gal.
--	--------------

<b>Future Storage Capacity Deficit</b>	(571,467) gal.
--	----------------

**Projected Required Storage Capacity at Buildout**

Residential ERUs

400 gpd	X	1,022 ERUs	=	408,800 gpd
ERU				

Commercial ERUs

400 gpd	X	998 ERUs	=	399,200 gpd
ERU				

Other ERUs

400 gpd	X	100 ERUs	=	40,000 gpd
ERU				

Fire Flow

3,500 gpm	X	60 min	X	3 hr =	630,000 gal.
		1 hr			

Emergency Supply

25% of required storage		369,500 gal.
-------------------------	--	--------------

<b>Total Required Storage</b>	1,847,500 gal.
-------------------------------	----------------

<b>Total Existing Storage Capacity</b>	700,000 gal.
--	--------------

<b>Future Storage Capacity Deficit</b>	(1,147,500) gal.
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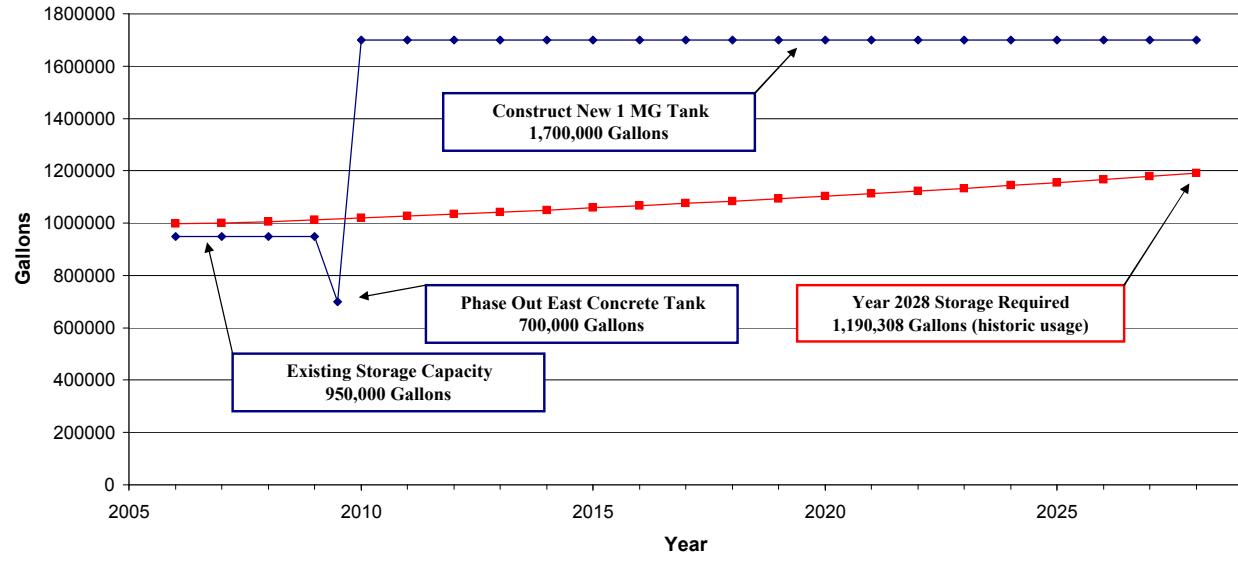
<b>Water Storage Capacity Summary</b>				
	<b>Historic</b>		<b>State Std.</b>	
Existing Water Storage Capacity	700,000	gal.	700,000	gal.
Existing Water Storage Surplus	(306,019)	gal.	(349,959)	gal.
Projected 2028 Water Storage Deficit*	(490,308)	gal.	(571,467)	gal.
Projected Buildout Water Storage Deficit*	(750,000)	gal.	(1,147,500)	gal.
<b>ERUs Serviceable at buildout with current storage capacity*</b>	<b>-3,000</b>		<b>-2,869</b>	

\*Existing Storage may or may not service new connections through the planning period and beyond. Proposed new development should be added to the existing water model and checked for servicability with the existing tanks.

## Water Storage Analysis

Year	Both				Historic Usage								State Standard Usage						
	Residential	Other	Existing	Fire Flow	Commercial	Residential	Commercial	Other	Emergency	20 Year	Surplus	Commercial	Residential	Commercial	Other	Emergency	20 Year	Surplus	
	ERUs	ERUs	Storage	Stg rqd	ERUs	Stg rqd	Stg rqd	Stg rqd	Supply	Stg rqd	Storage	ERUs	Stg rqd	Stg rqd	Stg rqd	Supply	Stg rqd	Storage	
2005	210	4	700,000	630,000	441	52,500	110,205	1,000	198,426	992,131	(292,131)	275	84,000	110,112	1,600	206,428	1,032,140	(332,140)	
2006	217	4	700,000	630,000	450	54,250	112,575	1,000	199,456	997,281	(297,281)	281	86,800	112,480	1,600	207,720	1,038,600	(338,600)	
2007	222	5	700,000	630,000	450	55,500	112,575	1,250	199,831	999,156	(299,156)	281	88,800	112,480	2,000	208,320	1,041,600	(341,600)	
2008	229	6	700,000	630,000	464	57,250	116,065	1,500	201,204	1,006,019	(306,019)	290	91,600	115,967	2,400	209,992	1,049,959	(349,959)	
2009	236	6	700,000	630,000	479	59,000	119,663	1,547	202,552	1,012,762	(312,762)	299	94,400	119,562	2,474	211,609	1,058,045	(358,045)	
2010	243	6	700,000	630,000	493	60,750	123,372	1,594	203,929	1,019,646	(319,646)	308	97,200	123,268	2,551	213,255	1,066,274	(366,274)	
2011	251	7	700,000	630,000	509	62,750	127,197	1,644	205,398	1,026,988	(326,988)	318	100,400	127,090	2,630	215,030	1,075,150	(375,150)	
2012	259	7	700,000	630,000	525	64,750	131,140	1,695	206,896	1,034,481	(334,481)	328	103,600	131,029	2,712	216,835	1,084,176	(384,176)	
2013	267	7	700,000	630,000	541	66,750	135,205	1,747	208,426	1,042,128	(342,128)	338	106,800	135,091	2,796	218,672	1,093,359	(393,359)	
2014	275	7	700,000	630,000	558	68,750	139,397	1,802	209,987	1,049,935	(349,935)	348	110,000	139,279	2,882	220,540	1,102,702	(402,702)	
2015	284	7	700,000	630,000	575	71,000	143,718	1,857	211,644	1,058,219	(358,219)	359	113,600	143,597	2,972	222,542	1,112,711	(412,711)	
2016	293	8	700,000	630,000	593	73,250	148,173	1,915	213,335	1,066,673	(366,673)	370	117,200	148,048	3,064	224,578	1,122,890	(422,890)	
2017	302	8	700,000	630,000	611	75,500	152,767	1,974	215,060	1,075,301	(375,301)	382	120,800	152,638	3,159	226,649	1,133,246	(433,246)	
2018	311	8	700,000	630,000	630	77,750	157,502	2,036	216,822	1,084,110	(384,110)	393	124,400	157,370	3,257	228,757	1,143,783	(443,783)	
2019	321	8	700,000	630,000	650	80,250	162,385	2,099	218,683	1,093,417	(393,417)	406	128,400	162,248	3,358	231,001	1,155,007	(455,007)	
2020	331	9	700,000	630,000	670	82,750	167,419	2,164	220,583	1,102,916	(402,916)	418	132,400	167,278	3,462	233,285	1,166,424	(466,424)	
2021	341	9	700,000	630,000	690	85,250	172,609	2,231	222,522	1,112,612	(412,612)	431	136,400	172,463	3,569	235,608	1,178,041	(478,041)	
2022	352	9	700,000	630,000	712	88,000	177,960	2,300	224,565	1,122,825	(422,825)	445	140,800	177,810	3,680	238,072	1,190,362	(490,362)	
2023	363	9	700,000	630,000	734	90,750	183,477	2,371	226,649	1,133,247	(433,247)	458	145,200	183,322	3,794	240,579	1,202,895	(502,895)	
2024	374	10	700,000	630,000	757	93,500	189,164	2,445	228,777	1,143,886	(443,886)	473	149,600	189,005	3,912	243,129	1,215,645	(515,645)	
2025	386	10	700,000	630,000	780	96,500	195,028	2,521	231,012	1,155,061	(455,061)	487	154,400	194,864	4,033	245,824	1,229,121	(529,121)	
2026	398	10	700,000	630,000	804	99,500	201,074	2,599	233,293	1,166,466	(466,466)	502	159,200	200,905	4,158	248,566	1,242,828	(542,828)	
2027	410	11	700,000	630,000	829	102,500	207,308	2,679	235,622	1,178,109	(478,109)	518	164,000	207,133	4,287	251,355	1,256,774	(556,774)	
2028	423	11	700,000	630,000	855	105,750	213,734	2,762	238,062	1,190,308	(490,308)	534	169,200	213,554	4,420	254,293	1,271,467	(571,467)	

Chart V.C.1 Town of Springdale Water Storage (3500 gpm Fire Flow x 3 hours)



## Water Distribution

### Historic Usage

Total Current ERUs = 699

#### A. Existing Distribution Requirement (Historical Usage):

Indoor Peak Instantaneous Demand:			
Q= 10.8 X N^.64		N= Number of ERU's	
Q= 10.8 X (699) ^^.64			= 714 gpm
Outdoor Peak Instantaneous Demand:			
- ERU. X 0.15 acre X	ERU	9.8 gpm	= - gpm
		irr. acre	
Current Peak Instantaneous Demand			
			= 714 gpm
Peak Day Demand & Fire Flow			
All ERUs			
699 ERU's X 500 gpd X	ERU	1 day : 24 hr	1 hr : 60 min.
			= 243 gpm
Outdoor Usage			
- ERU's X 110 gpd X	ERU	1 day : 24 hr	1 hr : 60 min.
			= 0 gpm
Fire Flow			= 3,500 gpm
Current Peak Day Demand + Fire Flow			
			= 3,743 gpm

B. Total Projected ERUs 20 Years = 1,289

#### Distribution Requirement for projected 20 year growth (Historic Usage):

Indoor Peak Instantaneous Demand:			
Q= 10.8 X N^.64		N= Number of ERU's	
Q= 10.8 X 1,289 ^^.64			= 1,057 gpm
Outdoor Peak Instantaneous Demand:			
- ERU. X 0.15 acre X	conn.	9.8 gpm	= - gpm
		irr. acre	
Projected Peak Instantaneous Demand			
			= 1,057 gpm
Peak Day Demand & Fire Flow			
All ERUs			
1,289 ERU's X 500 gpd X	ERU	1 day : 24 hr	1 hr : 60 min.
			= 448 gpm
Outdoor Usage			
- ERU's X 110 gpd X	ERU	1 day : 24 hr	1 hr : 60 min.
			= - gpm
Fire Flow			= 3,500 gpm
Projected Peak Day Demand + Fire Flow			
			= 3,948 gpm

C. Total Projected Buildout ERUs = 2,120

#### Distribution Requirement for projected buildout growth (Historic Usage):

Indoor Peak Instantaneous Demand:			
Q= 10.8 X N^.64		N= Number of ERU's	
Q= 10.8 X 2,120 ^^.64			= 1,453 gpm
Outdoor Peak Instantaneous Demand:			
- ERU. X 0.15 acre X	conn.	9.8 gpm	= - gpm
		irr. acre	
Projected Peak Instantaneous Demand			
			= 1,453 gpm
Peak Day Demand & Fire Flow			
All ERUs			
2,120 ERU's X 500 gpd X	ERU	1 day : 24 hr	1 hr : 60 min.
			= 736 gpm
Outdoor Usage			
- ERU's X 110 gpd X	ERU	1 day : 24 hr	1 hr : 60 min.
			= - gpm
Fire Flow			= 3,500 gpm
Projected Peak Day Demand + Fire Flow			
			= 4,236 gpm

## State Standard Usage

Total Current ERUs = 525

### A. Existing Distribution Requirement (State Standard Usage):

<b>Indoor Peak Instantaneous Demand:</b>	
$Q = 10.8 X N^{.64}$	N= Number of ERU's
$Q = 10.8 X (525)^{.64}$	= 595 gpm
<b>Outdoor Peak Instantaneous Demand:</b>	
- ERU. X $\frac{0.15 \text{ acre}}{\text{ERU}}$ $\frac{9.8 \text{ gpm}}{\text{irr. acre}}$	= - gpm
<b>Current Peak Instantaneous Demand</b>	
	= <b>595 gpm</b>
<b>Peak Day Demand &amp; Fire Flow</b>	
All ERUs	
525 ERU's X $\frac{800 \text{ gpd}}{\text{ERU}}$ $\frac{1 \text{ day}}{24 \text{ hr}}$ $\frac{1 \text{ hr}}{60 \text{ min.}}$	= 292 gpm
Outdoor Usage	
- ERU's X $\frac{110 \text{ gpd}}{\text{ERU}}$ $\frac{1 \text{ day}}{24 \text{ hr}}$ $\frac{1 \text{ hr}}{60 \text{ min.}}$	= 0 gpm
Fire Flow	= 3,500 gpm
<b>Current Peak Day Demand + Fire Flow</b>	
	= <b>3,792 gpm</b>

B. Total Projected ERUs 20 Years = 968

### Distribution Requirement for projected 20 year growth (State Standard Usage):

<b>Indoor Peak Instantaneous Demand:</b>	
$Q = 10.8 X N^{.64}$	N= Number of ERU's
$Q = 10.8 X 968^{.64}$	= 880 gpm
<b>Outdoor Peak Instantaneous Demand:</b>	
- ERU. X $\frac{0.15 \text{ acre}}{\text{conn.}}$ $\frac{9.8 \text{ gpm}}{\text{irr. acre}}$	= - gpm
<b>Projected Peak Instantaneous Demand</b>	
	= <b>880 gpm</b>
<b>Peak Day Demand &amp; Fire Flow</b>	
All ERUs	
968 ERU's X $\frac{800 \text{ gpd}}{\text{ERU}}$ $\frac{1 \text{ day}}{24 \text{ hr}}$ $\frac{1 \text{ hr}}{60 \text{ min.}}$	= 538 gpm
Outdoor Usage	
- ERU's X $\frac{110 \text{ gpd}}{\text{ERU}}$ $\frac{1 \text{ day}}{24 \text{ hr}}$ $\frac{1 \text{ hr}}{60 \text{ min.}}$	= - gpm
Fire Flow	= 3,500 gpm
<b>Projected Peak Day Demand + Fire Flow</b>	
	= <b>4,038 gpm</b>

C. Total Projected Buildout ERUs = 2,120

### Distribution Requirement for projected buildout growth (State Standard Usage):

<b>Indoor Peak Instantaneous Demand:</b>	
$Q = 10.8 X N^{.64}$	N= Number of ERU's
$Q = 10.8 X 2,120^{.64}$	= 1,453 gpm
<b>Outdoor Peak Instantaneous Demand:</b>	
- ERU. X $\frac{0.15 \text{ acre}}{\text{conn.}}$ $\frac{9.8 \text{ gpm}}{\text{irr. acre}}$	= - gpm
<b>Projected Peak Instantaneous Demand</b>	
	= <b>1,453 gpm</b>
<b>Peak Day Demand &amp; Fire Flow</b>	
All ERUs	
2,120 ERU's X $\frac{800 \text{ gpd}}{\text{ERU}}$ $\frac{1 \text{ day}}{24 \text{ hr}}$ $\frac{1 \text{ hr}}{60 \text{ min.}}$	= 1,178 gpm
Outdoor Usage	
- ERU's X $\frac{110 \text{ gpd}}{\text{ERU}}$ $\frac{1 \text{ day}}{24 \text{ hr}}$ $\frac{1 \text{ hr}}{60 \text{ min.}}$	= - gpm
Fire Flow	= 3,500 gpm
<b>Projected Peak Day Demand + Fire Flow</b>	
	= <b>4,678 gpm</b>

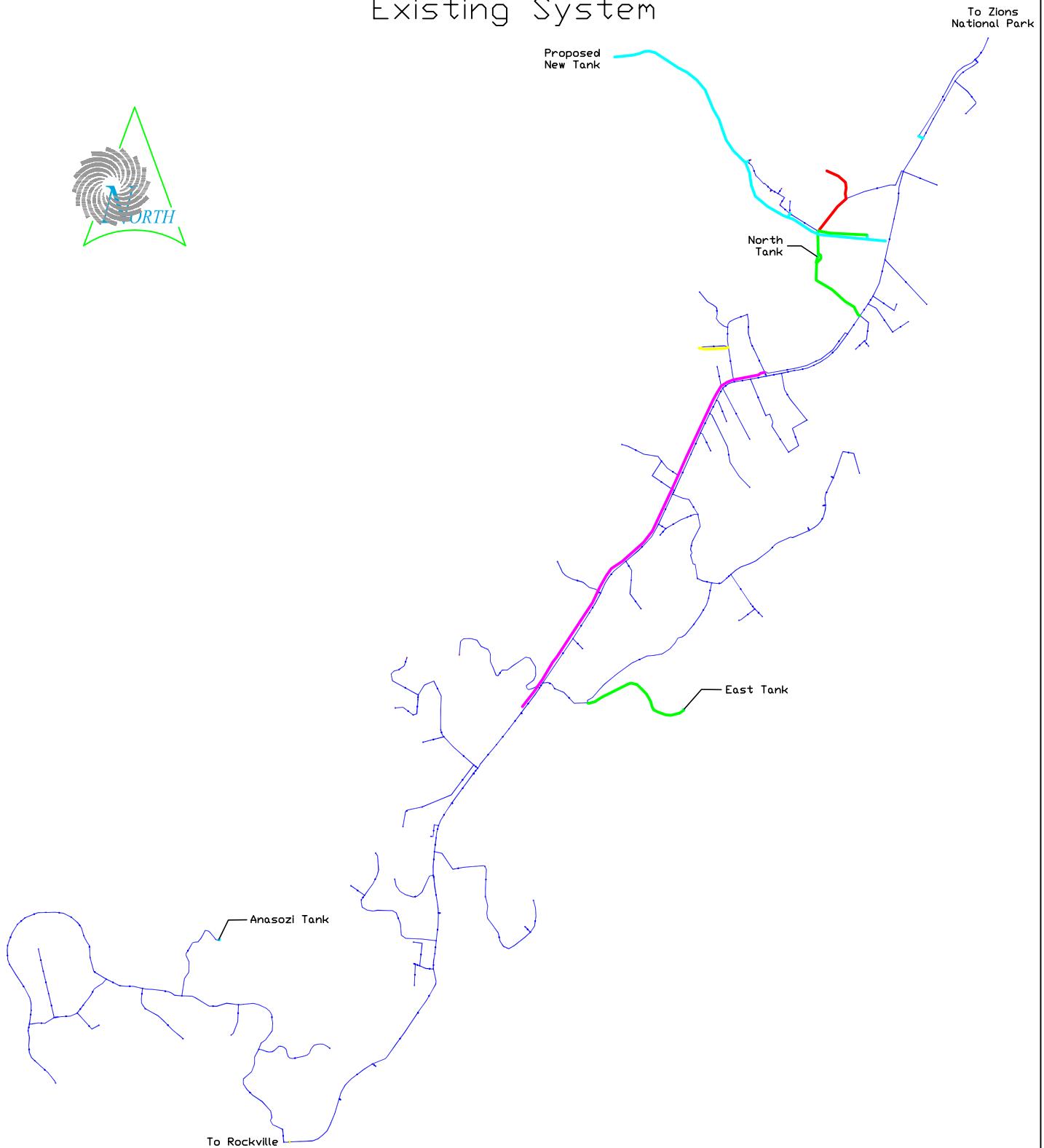


# APPENDIX C

## WATER DISTRIBUTION SYSTEM ANALYSIS

Page 3

# Springdale Culinary Water Master Plan Line Upgrades Existing System



Transite Line	—
Class 200 Line	—
2" Line	—
Abandoned Line	—
New Line	—



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# Springdale Culinary Water Master Plan

## Existing Peak Instantaneous Demand

### Existing System



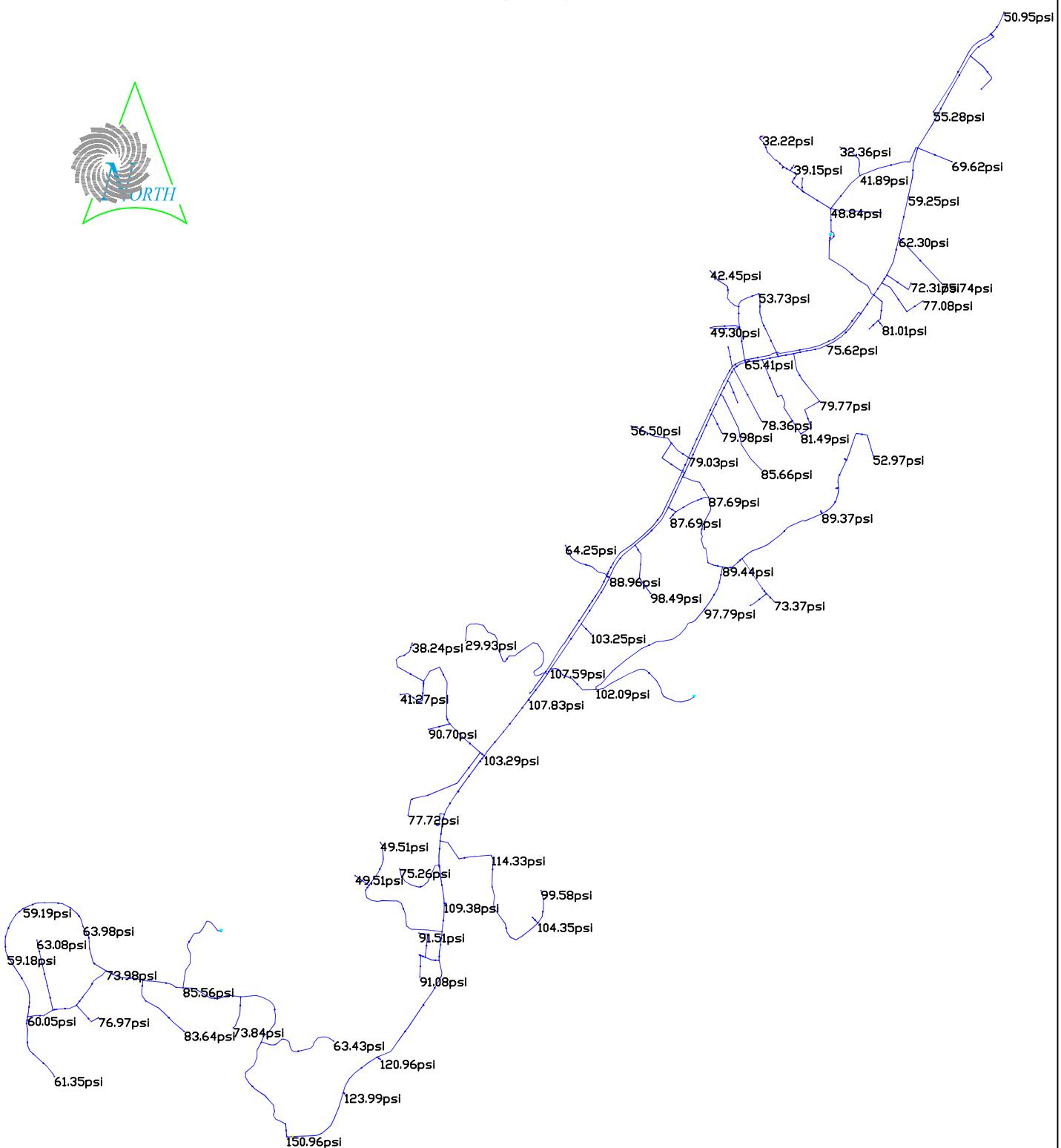
HEADLOSS  
0-5 ft        
5-10 ft        
10 + ft      



# Springdale Culinary Water Master Plan

## Future Peak Instantaneous Demand

### Existing System



HEADLOSS  
0-5 ft  
5-10 ft  
10 + ft

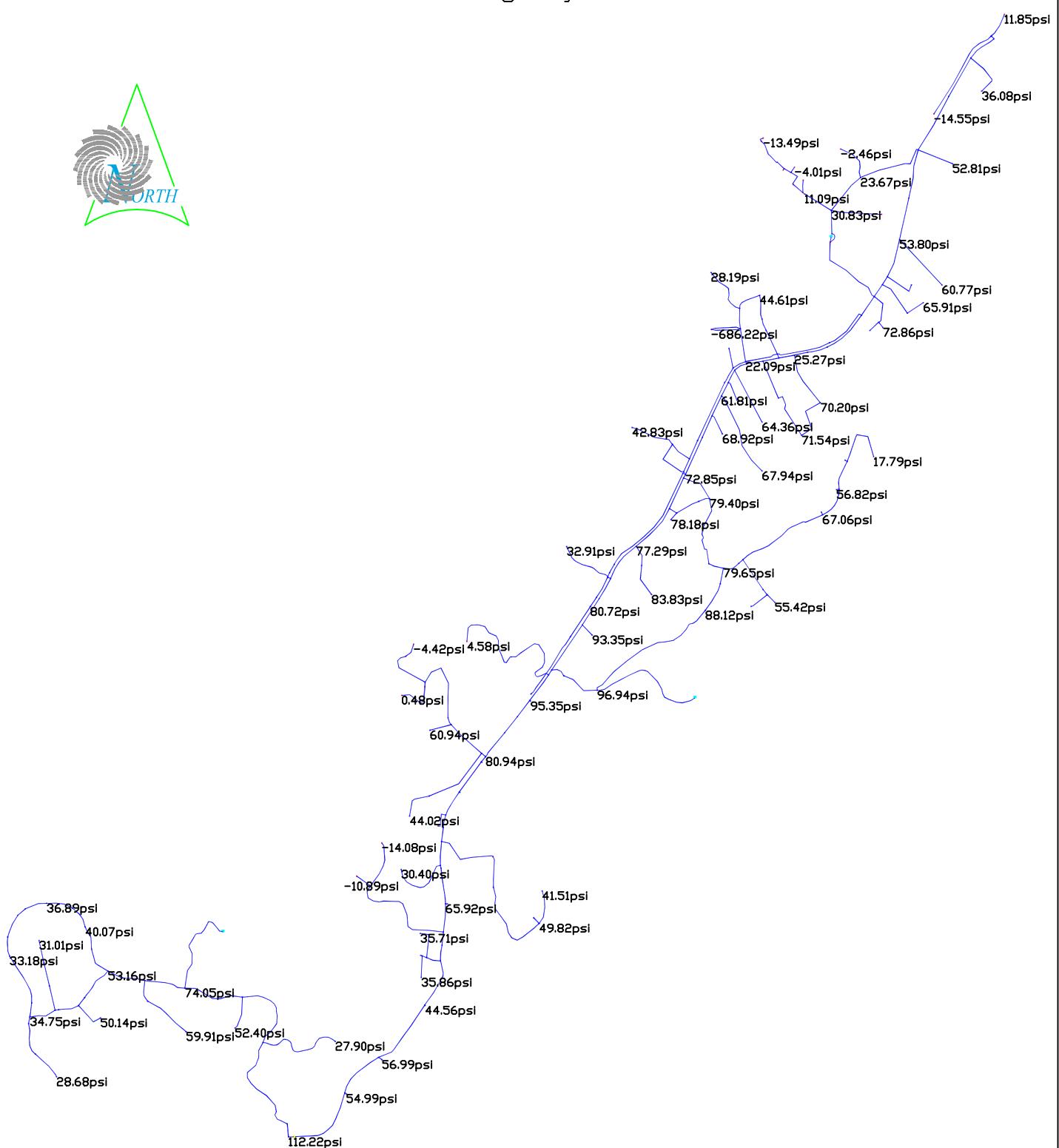
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# Springdale Culinary Water Master Plan

## Existing Fire Flow Demand

### Existing System



Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
1	J10	65.68	1,000.00	36.31	3,996.79	1,298.90
2	J100	42.45	1,000.00	29.43	4,032.93	1,395.48
3	J102	58.91	1,000.00	51.50	4,045.85	2,856.79
4	J104	49.30	1,000.00	-689.65	2,357.37	174.90
5	J106	66.63	1,000.00	59.57	4,046.49	3,367.22
6	J108	75.72	1,000.00	66.14	4,040.65	2,942.54
7	J112	70.49	1,000.00	63.12	4,045.68	3,407.14
8	J114	85.66	1,000.00	69.19	4,024.68	2,246.45
9	J116	74.78	1,000.00	67.21	4,045.10	3,499.76
10	J118	79.98	1,000.00	70.18	4,039.96	3,024.48
11	J12	58.75	1,000.00	34.21	4,007.94	1,314.80
12	J120	66.25	1,000.00	58.45	4,044.90	3,070.49
13	J122	76.48	1,000.00	66.03	4,038.39	2,794.76
14	J124	79.03	1,000.00	71.01	4,043.89	3,556.77
15	J126	80.33	1,000.00	71.84	4,042.79	3,441.53
16	J128	80.77	1,000.00	73.81	4,046.35	4,108.95
17	J130	81.20	1,000.00	74.18	4,046.20	4,102.90
18	J132	72.97	1,000.00	64.93	4,043.86	3,336.41
19	J134	56.50	1,000.00	43.76	4,033.00	1,902.48
20	J136	76.48	1,000.00	68.93	4,045.09	3,591.86
21	J138	87.69	1,000.00	80.26	4,045.23	4,148.39
22	J14	55.71	1,000.00	27.87	4,000.32	1,163.89
23	J140	87.69	1,000.00	80.02	4,044.68	4,035.87
24	J142	87.69	1,000.00	80.05	4,044.75	4,055.48
25	J144	84.65	1,000.00	77.22	4,045.22	4,041.35
26	J146	87.69	1,000.00	79.14	4,042.65	3,683.71

Date: Tuesday, May 27, 2008, Time: 11:12:52, Page 1

Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
27	J148	96.37	1,000.00	87.00	4,040.79	3,696.57
28	J150	89.44	1,000.00	79.85	4,040.29	3,439.42
29	J152	91.64	1,000.00	82.02	4,040.29	3,477.35
30	J154	97.79	1,000.00	88.28	4,040.75	3,654.05
31	J156	88.11	1,000.00	75.78	4,033.89	2,856.19
32	J158	102.09	1,000.00	97.14	4,052.18	6,431.38
33	J16	54.00	1,000.00	34.36	4,019.30	1,392.59
34	J160	107.59	1,000.00	100.38	4,045.67	5,123.22
35	J162	110.92	1,000.00	104.69	4,048.62	5,841.68
36	J164	77.27	1,000.00	61.04	4,024.87	2,140.29
37	J166	77.27	1,000.00	58.22	4,018.37	1,928.40
38	J168	75.11	1,000.00	58.15	4,023.21	2,034.22
39	J170	73.37	1,000.00	55.29	4,020.61	1,914.58
40	J172	89.37	1,000.00	66.60	4,009.71	1,938.26
41	J174	89.37	1,000.00	66.29	4,009.00	1,921.66
42	J176	82.87	1,000.00	56.25	4,000.81	1,663.76
43	J178	82.87	1,000.00	56.53	4,001.46	1,674.64
44	J18	59.22	1,000.00	46.51	4,035.33	1,983.22
45	J180	89.37	1,000.00	59.57	3,993.49	1,643.99
46	J182	89.37	1,000.00	59.85	3,994.11	1,653.22
47	J184	52.97	1,000.00	17.12	3,979.51	959.11
48	J186	86.82	1,000.00	75.14	4,035.40	2,929.81
49	J188	86.36	1,000.00	78.08	4,043.20	3,756.41
50	J190	86.36	1,000.00	78.01	4,043.03	3,739.48
51	J192	86.79	1,000.00	78.72	4,043.68	3,874.99
52	J194	92.86	1,000.00	82.38	4,038.11	3,293.66

Date: Tuesday, May 27, 2008, Time: 11:12:52, Page 2

Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
53	J196	98.49	1,000.00	84.57	4,030.17	2,808.30
54	J198	88.96	1,000.00	81.21	4,044.41	4,093.03
55	J20	69.62	1,000.00	53.04	4,026.40	1,913.82
56	J200	88.52	1,000.00	80.63	4,044.08	4,014.07
57	J202	88.52	1,000.00	80.60	4,044.01	4,000.13
58	J204	64.25	1,000.00	33.46	3,991.22	1,238.24
59	J206	96.75	1,000.00	81.55	4,027.21	2,632.69
60	J208	94.16	1,000.00	86.15	4,043.82	4,145.97
61	J210	92.85	1,000.00	82.45	4,038.29	3,337.37
62	J212	99.35	1,000.00	91.31	4,043.73	4,300.04
63	J214	101.08	1,000.00	70.35	3,991.35	1,750.43
64	J216	103.25	1,000.00	93.73	4,040.31	3,839.51
65	J218	107.49	1,000.00	99.30	4,043.17	4,691.43
66	J22	41.89	1,000.00	22.95	4,020.97	1,097.22
67	J220	29.93	1,000.00	4.20	4,002.69	569.92
68	J222	107.57	1,000.00	47.34	3,923.24	1,240.26
69	J224	107.83	1,000.00	94.16	4,029.31	3,345.85
70	J226	107.83	1,000.00	94.27	4,029.56	3,368.98
71	J228	104.91	1,000.00	87.98	4,021.04	2,858.31
72	J230	103.30	1,000.00	82.97	4,012.49	2,511.57
73	J232	103.29	1,000.00	77.90	3,999.77	2,177.46
74	J234	101.99	1,000.00	75.96	3,998.32	2,117.89
75	J236	103.67	1,000.00	77.19	3,997.15	2,123.00
76	J238	103.67	1,000.00	77.27	3,997.34	2,128.12
77	J24	32.36	1,000.00	-3.29	3,982.42	546.86
78	J240	90.26	1,000.00	59.43	3,987.17	1,703.93

Date: Tuesday, May 27, 2008, Time: 11:12:52, Page 3

## Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
79	J242	90.70	1,000.00	57.57	3,981.87	1,625.41
80	J244	88.96	1,000.00	57.26	3,985.16	1,651.96
81	J246	41.27	1,000.00	-3.26	3,955.47	628.63
82	J248	58.61	1,000.00	16.26	3,960.54	950.10
83	J250	71.17	1,000.00	30.80	3,965.08	1,168.13
84	J252	72.04	1,000.00	32.14	3,966.17	1,189.39
85	J254	41.27	1,000.00	-3.56	3,954.78	626.10
86	J256	38.24	1,000.00	-8.19	3,951.10	554.86
87	J258	77.72	1,000.00	40.81	3,973.18	1,330.32
88	J26	48.84	1,000.00	29.66	4,020.44	1,282.02
89	J260	98.95	1,000.00	65.51	3,981.20	1,722.71
90	J262	105.13	1,000.00	73.21	3,983.97	1,896.49
91	J264	105.13	1,000.00	73.29	3,984.14	1,899.79
92	J266	105.32	1,000.00	67.81	3,970.50	1,708.59
93	J268	106.26	1,000.00	70.56	3,974.84	1,776.99
94	J270	102.35	1,000.00	9.69	3,843.36	939.97
95	J272	114.33	1,000.00	68.15	3,950.27	1,577.08
96	J274	105.15	1,000.00	62.13	3,957.40	1,559.18
97	J276	105.67	1,000.00	66.18	3,965.73	1,656.07
98	J278	105.15	1,000.00	62.48	3,958.20	1,567.47
99	J28	57.91	1,000.00	-4,473.10	-6,393.33	76.33
100	J280	105.22	1,000.00	55.70	3,942.56	1,413.91
101	J282	100.01	1,000.00	40.11	3,918.56	1,202.67
102	J284	99.58	1,000.00	36.92	3,912.21	1,165.28
103	J286	104.35	1,000.00	45.25	3,920.44	1,252.73
104	J288	75.26	1,000.00	26.19	3,943.44	1,086.04

Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
105	J290	109.38	1,000.00	61.51	3,945.96	1,497.62
106	J292	109.38	1,000.00	61.71	3,946.42	1,501.94
107	J294	98.47	1,000.00	45.63	3,934.31	1,294.51
108	J296	102.38	1,000.00	51.14	3,938.01	1,361.55
109	J298	77.67	1,000.00	15.15	3,911.96	957.08
110	J300	78.10	1,000.00	14.99	3,910.59	955.98
111	J302	49.51	1,000.00	-15.39	3,906.49	607.66
112	J304	49.51	1,000.00	-18.60	3,899.07	591.23
113	J306	91.08	1,000.00	32.02	3,919.89	1,131.47
114	J308	91.51	1,000.00	34.53	3,924.69	1,162.53
115	J310	91.95	1,000.00	35.14	3,925.09	1,169.25
116	J312	92.38	1,000.00	36.08	3,926.27	1,180.61
117	J314	95.85	1,000.00	41.14	3,929.94	1,238.77
118	J316	91.51	1,000.00	31.87	3,918.56	1,128.69
119	J318	100.60	1,000.00	40.30	3,917.00	1,207.76
120	J32	74.51	1,000.00	71.35	4,057.68	6,569.61
121	J320	100.60	1,000.00	40.35	3,917.13	1,208.46
122	J322	119.22	1,000.00	51.22	3,899.20	1,270.49
123	J324	120.96	1,000.00	52.53	3,898.23	1,278.84
124	J326	123.99	1,000.00	50.46	3,886.45	1,243.89
125	J328	123.99	1,000.00	50.63	3,886.85	1,245.76
126	J330	150.96	1,000.00	109.16	4,030.92	1,920.83
127	J336	80.33	1,000.00	51.13	4,060.01	1,533.99
128	J338	63.43	1,000.00	24.93	4,038.53	1,079.05
129	J34	59.25	1,000.00	48.89	4,040.84	2,256.51
130	J340	76.01	1,000.00	55.29	4,079.60	1,811.94

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## Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
131	J342	73.84	1,000.00	49.69	4,071.68	1,609.07
132	J344	85.56	1,000.00	71.61	4,095.27	2,560.22
133	J346	76.71	1,000.00	57.13	4,081.85	1,919.27
134	J348	83.64	1,000.00	56.60	4,064.62	1,666.44
135	J350	76.97	1,000.00	45.96	4,055.06	1,449.25
136	J352	70.90	1,000.00	43.04	4,062.34	1,451.35
137	J354	60.92	1,000.00	32.08	4,060.04	1,246.20
138	J356	63.08	1,000.00	26.67	4,042.54	1,113.17
139	J358	60.05	1,000.00	30.48	4,058.33	1,211.07
140	J36	62.30	1,000.00	54.38	4,046.50	2,803.86
141	J360	61.35	1,000.00	24.32	4,041.13	1,075.47
142	J362	73.98	1,000.00	49.27	4,069.71	1,618.82
143	J364	141.42	1,000.00	100.99	4,034.08	1,877.85
144	J366	75.41	1,000.00	53.52	4,076.52	1,759.10
145	J368	69.69	1,000.00	47.02	4,074.52	1,623.54
146	J370	76.54	1,000.00	49.20	4,063.54	1,562.89
147	J372	63.09	1,000.00	34.38	4,060.34	1,289.15
148	J374	66.55	1,000.00	35.03	4,053.86	1,273.84
149	J376	68.28	1,000.00	34.29	4,048.13	1,242.21
150	J378	63.95	1,000.00	34.12	4,057.75	1,273.93
151	J38	75.74	1,000.00	61.31	4,031.50	2,202.34
152	J380	59.18	1,000.00	28.92	4,056.75	1,178.01
153	J382	50.52	1,000.00	20.29	4,056.82	1,012.72
154	J384	53.99	1,000.00	23.94	4,057.25	1,085.39
155	J386	59.19	1,000.00	29.27	4,057.54	1,186.11
156	J388	62.23	1,000.00	32.70	4,058.48	1,250.86

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## Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
157	J390	63.97	1,000.00	35.14	4,060.09	1,300.37
158	J392	63.98	1,000.00	35.98	4,062.05	1,324.50
159	J394	67.03	1,000.00	40.17	4,064.71	1,414.89
160	J396	57.88	1,000.00	27.56	4,056.60	1,152.93
161	J398	54.42	1,000.00	20.68	4,048.72	1,019.00
162	J40	66.24	1,000.00	61.04	4,052.88	3,963.51
163	J400	13.78	1,000.00	13.41	4,065.94	-7,216.56
164	J402	4.82	1,000.00	-12.74	4,024.61	-1,116.24
165	J404	43.09	1,000.00	8.93	3,985.60	798.63
166	J406	40.37	1,000.00	-1,109.23	1,411.05	112.77
167	J408	39.15	1,000.00	-6.63	3,958.71	602.84
168	J410	39.15	1,000.00	-4.91	3,962.67	615.57
169	J412	39.15	1,000.00	-3.88	3,965.05	623.92
170	J414	39.15	1,000.00	-4.73	3,963.07	616.92
171	J416	35.25	1,000.00	-9.65	3,960.74	530.78
172	J418	28.32	1,000.00	-19.52	3,953.95	353.15
173	J42	72.31	1,000.00	63.65	4,044.89	2,918.54
174	J420	32.22	1,000.00	-16.18	3,952.66	445.08
175	J424	41.79	1,000.00	6.53	3,983.07	757.18
176	J426	39.16	1,000.00	-1.91	3,969.58	641.14
177	J428	55.28	1,000.00	-14.80	3,902.84	685.55
178	J430	55.71	1,000.00	27.46	3,999.37	1,153.83
179	J432	50.95	1,000.00	11.80	3,974.22	879.95
180	J44	71.45	1,000.00	67.01	4,054.66	4,756.46
181	J46	77.52	1,000.00	68.98	4,045.19	3,100.25
182	J48	77.08	1,000.00	66.54	4,040.56	2,687.23

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Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
183	J52	81.01	1,000.00	73.83	4,048.38	3,569.82
184	J54	81.44	1,000.00	73.68	4,047.05	3,399.55
185	J56	79.71	1,000.00	73.22	4,049.98	3,787.37
186	J58	77.15	1,000.00	72.53	4,053.40	5,143.09
187	J60	75.62	1,000.00	69.59	4,049.60	4,035.44
188	J62	72.49	1,000.00	66.42	4,049.28	3,923.29
189	J64	69.83	3,500.00	28.38	3,967.51	3,907.98
190	J66	77.39	1,000.00	71.32	4,049.61	4,073.35
191	J68	66.77	1,000.00	60.97	4,049.71	3,895.99
192	J70	79.77	1,000.00	71.61	4,044.27	3,395.34
193	J72	81.49	1,000.00	72.94	4,043.33	3,335.90
194	J74	74.11	1,000.00	66.27	4,044.95	3,304.22
195	J76	66.74	1,000.00	60.66	4,049.00	3,735.19
196	J78	67.63	1,000.00	61.82	4,049.66	3,924.62
197	J80	65.84	1,000.00	59.90	4,049.23	3,793.28
198	J82	65.41	3,500.00	25.20	3,970.16	3,767.81
199	J84	64.96	1,000.00	58.06	4,047.00	3,321.31
200	J86	59.33	1,000.00	50.34	4,042.17	2,497.40
201	J88	78.36	1,000.00	65.65	4,033.51	2,476.91
202	J90	66.23	1,000.00	59.64	4,047.65	3,527.20
203	J92	61.98	1,000.00	54.81	4,046.49	3,064.85
204	J94	61.97	1,000.00	54.46	4,045.69	2,956.94
205	J96	53.73	1,000.00	45.89	4,044.91	2,512.38
206	J98	52.85	1,000.00	45.07	4,045.02	2,487.88

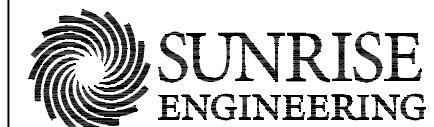
# Springdale Culinary Water Master Plan

## Future Peak Instantaneous Demands With Recommended Improvements



# Springdale Culinary Water Master Plan

## Future Fire Flow Demands With Recommended Improvements



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Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
1	J10	90.97	1,000.00	60.22	4,051.97	1,639.63
2	J100	67.79	1,000.00	48.76	4,077.54	1,774.46
3	J102	84.20	1,000.00	73.57	4,096.78	3,389.28
4	J104	74.67	1,000.00	61.09	4,089.99	2,470.44
5	J106	91.79	1,000.00	80.06	4,093.77	3,392.77
6	J108	100.89	1,000.00	86.42	4,087.45	3,025.39
7	J112	95.64	1,000.00	83.40	4,092.47	3,380.75
8	J114	110.80	1,000.00	83.73	4,058.24	2,062.37
9	J116	99.88	1,000.00	87.06	4,090.92	3,372.89
10	J118	105.08	1,000.00	90.04	4,085.80	3,046.51
11	J12	84.04	1,000.00	59.85	4,067.13	1,797.81
12	J120	91.38	1,000.00	80.55	4,095.90	3,675.24
13	J122	101.59	1,000.00	89.83	4,093.31	3,752.90
14	J124	104.13	1,000.00	92.17	4,092.72	3,784.06
15	J126	105.41	1,000.00	93.35	4,092.45	3,802.50
16	J128	105.81	1,000.00	93.28	4,091.27	3,687.01
17	J130	106.22	1,000.00	93.53	4,090.85	3,658.73
18	J132	98.05	1,000.00	85.00	4,090.17	3,318.38
19	J134	81.59	1,000.00	61.72	4,074.44	2,038.21
20	J136	101.56	1,000.00	88.63	4,090.55	3,410.33
21	J138	112.68	1,000.00	99.14	4,088.80	3,617.64
22	J14	81.01	1,000.00	55.71	4,064.57	1,698.27
23	J140	112.68	1,000.00	99.00	4,088.48	3,578.17
24	J142	112.68	1,000.00	99.19	4,088.92	3,622.33
25	J144	109.65	1,000.00	96.41	4,089.50	3,611.71
26	J146	112.68	1,000.00	98.29	4,086.83	3,408.02

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Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
27	J148	121.23	1,000.00	103.66	4,079.23	3,096.07
28	J150	114.29	1,000.00	96.41	4,078.51	2,930.23
29	J152	116.45	1,000.00	98.45	4,078.20	2,949.37
30	J154	122.52	1,000.00	104.44	4,078.04	3,043.32
31	J156	112.97	1,000.00	92.44	4,072.33	2,615.99
32	J158	126.42	1,000.00	111.07	4,084.35	3,558.94
33	J16	79.28	1,000.00	57.41	4,072.49	1,833.23
34	J160	132.49	1,000.00	118.54	4,087.57	4,028.35
35	J162	135.52	1,000.00	120.77	4,085.72	3,879.33
36	J164	102.13	1,000.00	77.76	4,063.46	2,138.45
37	J166	102.13	1,000.00	74.96	4,056.99	1,980.59
38	J168	99.97	1,000.00	74.88	4,061.82	2,061.88
39	J170	98.23	1,000.00	72.03	4,059.23	1,973.04
40	J172	114.24	1,000.00	83.38	4,048.43	1,981.82
41	J174	114.24	1,000.00	83.07	4,047.72	1,968.41
42	J176	107.73	1,000.00	73.04	4,039.56	1,759.19
43	J178	107.73	1,000.00	73.32	4,040.20	1,768.55
44	J18	84.50	1,000.00	71.24	4,092.42	2,717.74
45	J180	114.23	1,000.00	76.35	4,032.20	1,733.77
46	J182	114.23	1,000.00	76.62	4,032.83	1,741.86
47	J184	77.83	1,000.00	30.44	4,010.26	1,130.99
48	J186	111.67	1,000.00	91.77	4,073.79	2,654.60
49	J188	111.34	1,000.00	97.31	4,087.58	3,470.60
50	J190	111.33	1,000.00	98.19	4,089.62	3,708.31
51	J192	111.76	1,000.00	98.60	4,089.57	3,722.29
52	J194	117.84	1,000.00	101.62	4,082.54	3,183.72

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Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
53	J196	123.47	1,000.00	103.83	4,074.63	2,823.02
54	J198	113.92	1,000.00	100.69	4,089.39	3,760.11
55	J20	94.90	1,000.00	77.79	4,083.53	2,460.78
56	J200	113.48	1,000.00	100.33	4,089.55	3,768.70
57	J202	113.49	1,000.00	100.33	4,089.55	3,768.13
58	J204	89.22	1,000.00	70.31	4,076.26	2,295.01
59	J206	121.70	1,000.00	108.05	4,088.37	3,836.29
60	J208	119.10	1,000.00	104.81	4,086.89	3,611.53
61	J210	117.81	1,000.00	104.27	4,088.65	3,775.57
62	J212	124.28	1,000.00	109.51	4,085.73	3,616.79
63	J214	126.02	1,000.00	112.13	4,087.78	3,879.27
64	J216	128.18	1,000.00	111.93	4,082.32	3,396.40
65	J218	132.49	1,000.00	118.59	4,087.70	4,049.30
66	J22	67.30	1,000.00	57.06	4,099.69	2,716.85
67	J220	54.92	1,000.00	21.11	4,041.72	1,024.53
68	J222	132.48	1,000.00	118.19	4,086.77	3,943.37
69	J224	133.34	1,000.00	118.03	4,084.39	3,712.19
70	J226	133.34	1,000.00	118.13	4,084.64	3,736.36
71	J228	130.74	1,000.00	115.28	4,084.04	3,631.19
72	J230	129.44	1,000.00	114.02	4,084.14	3,618.36
73	J232	129.87	1,000.00	115.11	4,085.66	3,788.23
74	J234	128.57	1,000.00	113.80	4,085.62	3,757.36
75	J236	130.30	1,000.00	115.16	4,084.78	3,707.66
76	J238	130.30	1,000.00	115.25	4,084.97	3,726.45
77	J24	57.76	1,000.00	43.43	4,090.24	1,835.59
78	J240	116.85	1,000.00	97.40	4,074.78	2,788.97

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Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
79	J242	117.29	1,000.00	95.55	4,069.51	2,564.64
80	J244	115.55	1,000.00	95.24	4,072.81	2,676.89
81	J246	67.87	1,000.00	32.48	4,037.95	1,207.08
82	J248	85.20	1,000.00	51.99	4,042.99	1,513.33
83	J250	97.77	1,000.00	66.51	4,047.50	1,748.92
84	J252	98.64	1,000.00	67.84	4,048.58	1,778.13
85	J254	67.87	1,000.00	32.19	4,037.28	1,201.00
86	J256	64.84	1,000.00	27.57	4,033.62	1,123.71
87	J258	104.30	1,000.00	78.72	4,060.67	2,083.07
88	J26	74.26	1,000.00	66.88	4,106.34	3,993.07
89	J260	125.53	1,000.00	103.41	4,068.65	2,641.37
90	J262	132.04	1,000.00	115.40	4,081.32	3,438.96
91	J264	132.04	1,000.00	115.47	4,081.49	3,452.28
92	J266	132.47	1,000.00	114.54	4,078.34	3,235.44
93	J268	133.34	1,000.00	115.82	4,079.29	3,314.62
94	J270	129.43	1,000.00	55.19	3,948.37	1,251.52
95	J272	141.57	1,000.00	116.65	4,062.21	2,641.94
96	J274	132.48	1,000.00	113.45	4,075.82	3,074.92
97	J276	132.91	1,000.00	114.54	4,077.35	3,179.59
98	J278	132.48	1,000.00	113.51	4,075.96	3,083.53
99	J28	83.66	1,000.00	63.20	4,075.85	1,971.49
100	J280	132.46	1,000.00	104.26	4,054.62	2,316.35
101	J282	127.26	1,000.00	88.79	4,030.90	1,838.80
102	J284	126.83	1,000.00	85.61	4,024.58	1,756.60
103	J286	131.59	1,000.00	93.93	4,032.78	1,907.13
104	J288	102.59	1,000.00	77.24	4,061.26	2,068.91

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## Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
105	J290	136.84	1,000.00	116.84	4,073.65	3,016.01
106	J292	136.84	1,000.00	117.04	4,074.10	3,039.39
107	J294	126.06	1,000.00	105.56	4,072.62	2,785.37
108	J296	129.94	1,000.00	109.63	4,073.00	2,869.16
109	J298	105.23	1,000.00	73.82	4,047.38	1,826.40
110	J300	105.66	1,000.00	73.67	4,046.03	1,810.13
111	J302	77.07	1,000.00	43.30	4,041.94	1,376.58
112	J304	77.07	1,000.00	40.10	4,034.55	1,301.10
113	J306	118.71	1,000.00	93.72	4,062.28	2,311.66
114	J308	119.15	1,000.00	96.22	4,067.07	2,463.83
115	J310	119.58	1,000.00	96.83	4,067.46	2,483.54
116	J312	120.01	1,000.00	97.76	4,068.63	2,530.33
117	J314	123.48	1,000.00	102.79	4,072.22	2,720.86
118	J316	119.15	1,000.00	93.57	4,060.95	2,280.92
119	J318	128.37	1,000.00	107.23	4,071.47	2,734.89
120	J32	99.09	1,000.00	88.66	4,097.62	3,831.66
121	J320	128.37	1,000.00	107.28	4,071.60	2,739.98
122	J322	147.21	1,000.00	125.92	4,071.61	2,963.95
123	J324	148.94	1,000.00	127.23	4,070.64	2,945.61
124	J326	152.14	1,000.00	130.80	4,071.86	3,007.53
125	J328	152.14	1,000.00	130.97	4,072.26	3,025.07
126	J330	148.55	1,000.00	128.10	4,074.64	3,022.10
127	J336	78.50	1,000.00	60.68	4,082.03	2,073.80
128	J338	61.60	1,000.00	31.30	4,053.25	1,210.91
129	J34	84.50	1,000.00	73.45	4,097.51	3,122.02
130	J340	74.77	1,000.00	60.95	4,092.66	2,344.66

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## Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
131	J342	72.61	1,000.00	53.69	4,080.90	1,852.03
132	J344	84.82	1,000.00	75.15	4,103.44	3,286.69
133	J346	76.07	1,000.00	61.20	4,091.25	2,272.73
134	J348	83.00	1,000.00	60.71	4,074.12	1,856.93
135	J350	76.41	1,000.00	50.65	4,065.89	1,605.93
136	J352	70.35	1,000.00	47.72	4,073.14	1,633.35
137	J354	60.38	1,000.00	36.81	4,070.96	1,396.23
138	J356	62.54	1,000.00	31.47	4,053.62	1,211.68
139	J358	59.51	1,000.00	35.23	4,069.31	1,352.42
140	J36	87.40	1,000.00	78.26	4,101.62	3,809.36
141	J360	60.81	1,000.00	29.13	4,052.23	1,168.56
142	J362	73.40	1,000.00	53.77	4,080.10	1,850.64
143	J364	139.07	1,000.00	118.78	4,075.13	2,903.81
144	J366	74.77	1,000.00	57.61	4,085.96	2,031.45
145	J368	69.09	1,000.00	51.36	4,084.52	1,877.09
146	J370	75.98	1,000.00	53.85	4,074.27	1,762.86
147	J372	62.54	1,000.00	39.10	4,071.25	1,445.30
148	J374	66.01	1,000.00	39.80	4,064.86	1,410.65
149	J376	67.74	1,000.00	39.07	4,059.18	1,363.08
150	J378	63.41	1,000.00	38.88	4,068.73	1,421.17
151	J38	100.83	1,000.00	85.22	4,086.68	2,732.04
152	J380	58.64	1,000.00	33.67	4,067.70	1,310.46
153	J382	49.98	1,000.00	25.03	4,067.76	1,124.58
154	J384	53.44	1,000.00	28.66	4,068.15	1,207.05
155	J386	58.64	1,000.00	33.98	4,068.43	1,320.69
156	J388	61.68	1,000.00	37.40	4,069.32	1,395.17

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Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
157	J390	63.42	1,000.00	39.80	4,070.86	1,454.50
158	J392	63.42	1,000.00	40.62	4,072.73	1,486.87
159	J394	66.46	1,000.00	44.76	4,075.29	1,597.38
160	J396	57.34	1,000.00	32.33	4,067.60	1,282.77
161	J398	53.87	1,000.00	25.47	4,059.78	1,117.72
162	J40	90.99	1,000.00	80.72	4,098.29	3,598.98
163	J404	68.68	1,000.00	61.74	4,107.49	3,862.29
164	J406	66.05	4.75	65.89	4,123.06	178.42
165	J408	65.02	1,000.00	54.62	4,100.05	2,510.24
166	J410	65.05	1,000.00	57.36	4,106.38	3,213.03
167	J412	65.02	1,000.00	57.36	4,106.37	3,225.62
168	J414	65.05	1,000.00	57.53	4,106.78	3,280.31
169	J416	61.18	1,000.00	53.92	4,107.44	3,194.95
170	J418	54.37	1,000.00	48.46	4,110.84	3,467.08
171	J42	97.05	1,000.00	83.34	4,090.34	2,956.69
172	J420	58.27	1,000.00	51.80	4,109.55	3,376.57
173	J424	67.41	1,000.00	60.15	4,106.82	3,589.85
174	J426	64.96	1,000.00	57.12	4,105.83	3,166.74
175	J428	80.58	1,000.00	60.03	4,075.54	1,933.26
176	J430	81.01	1,000.00	55.59	4,064.30	1,693.16
177	J432	76.24	1,000.00	39.98	4,039.26	1,297.23
178	J434	86.23	1,000.00	77.96	4,103.92	4,112.74
179	J438	67.38	1,000.00	60.64	4,107.95	3,952.66
180	J44	96.13	1,000.00	85.77	4,097.94	3,740.07
181	J440	90.97	1,000.00	65.02	4,063.07	1,824.62
182	J442	83.63	1,000.00	70.81	4,093.43	2,762.79

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## Print Report

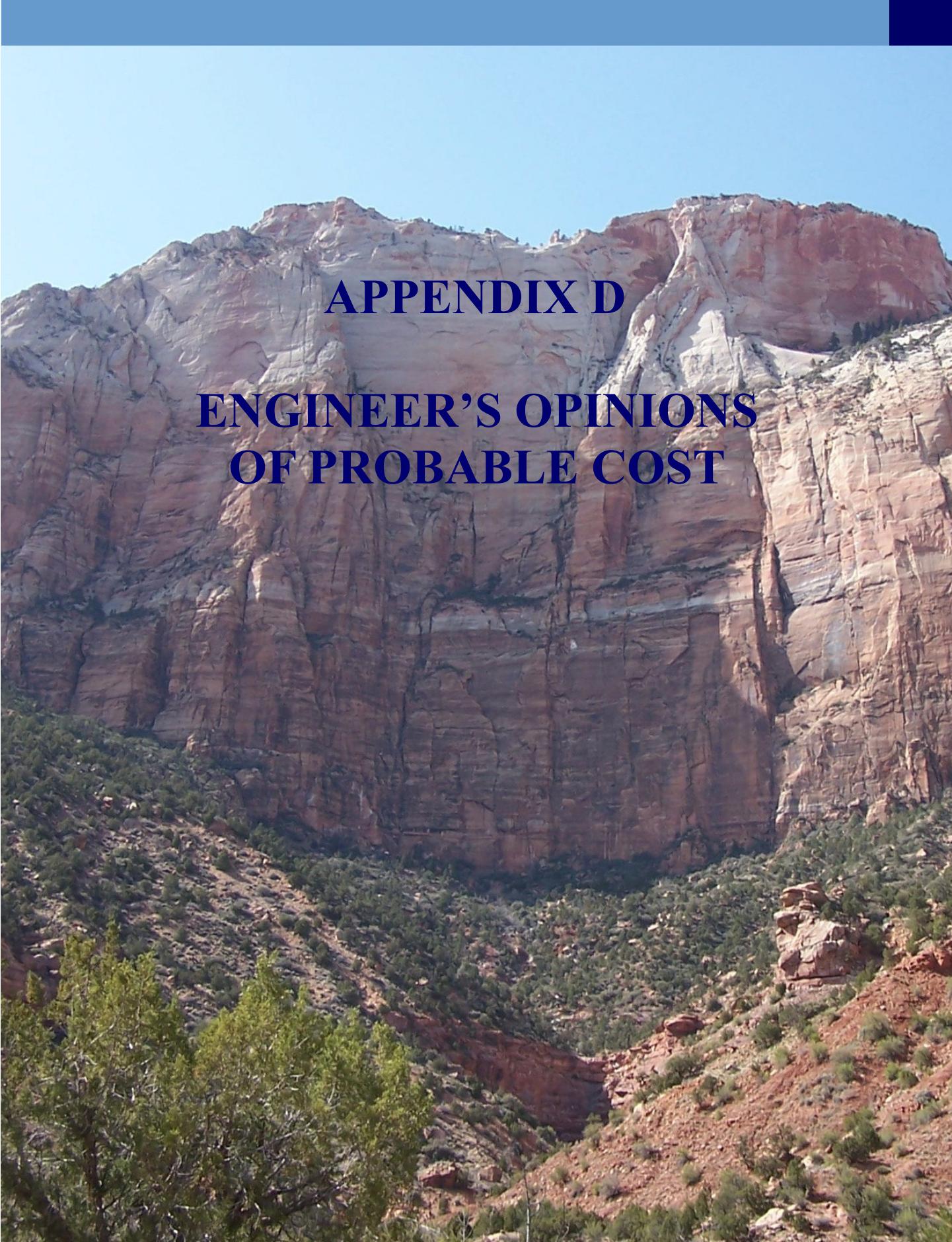
	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
183	J444	85.36	1,000.00	76.65	4,102.89	3,872.61
184	J446	66.55	1,000.00	59.88	4,108.18	3,934.67
185	J448	89.74	1,000.00	79.59	4,098.67	3,584.38
186	J450	101.69	1,000.00	90.07	4,094.87	3,545.11
187	J452	104.29	1,000.00	89.35	4,087.20	2,947.91
188	J454	104.19	1,000.00	93.56	4,096.92	3,963.83
189	J456	95.02	1,000.00	84.44	4,096.87	3,756.82
190	J458	95.87	1,000.00	84.84	4,095.79	3,646.24
191	J46	102.20	1,000.00	87.77	4,088.56	2,964.26
192	J460	100.63	1,000.00	88.62	4,093.51	3,495.47
193	J462	94.55	1,000.00	83.41	4,095.50	3,586.14
194	J464	72.13	1,000.00	60.04	4,093.57	2,642.69
195	J466	65.19	1,000.00	47.85	4,081.43	1,829.97
196	J468	93.09	1,000.00	81.14	4,093.26	3,369.97
197	J470	103.87	1,000.00	88.35	4,084.90	2,934.21
198	J472	107.34	1,000.00	89.33	4,079.15	2,678.60
199	J474	98.61	1,000.00	85.91	4,091.27	3,362.16
200	J476	101.09	1,000.00	88.17	4,090.48	3,430.42
201	J478	92.85	1,000.00	77.67	4,085.26	2,781.68
202	J48	101.76	1,000.00	85.34	4,083.95	2,681.92
203	J484	107.35	1,000.00	87.97	4,075.02	2,605.85
204	J486	89.58	1,000.00	65.54	4,064.25	1,940.24
205	J488	85.63	1,000.00	39.07	4,012.17	1,231.90
206	J490	122.56	1,000.00	105.75	4,081.05	3,223.71
207	J492	106.01	1,000.00	77.62	4,054.13	1,979.76
208	J494	88.36	1,000.00	69.96	4,080.46	2,227.83

Print Report

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
209	J496	53.36	1,000.00	26.20	4,060.46	1,135.67
210	J498	75.09	1,000.00	57.60	4,082.94	2,025.41
211	J500	65.09	1,000.00	50.11	4,089.65	1,984.21
212	J502	71.31	1,000.00	54.03	4,084.69	1,937.39
213	J504	76.64	1,000.00	63.81	4,095.26	2,510.05
214	J506	74.26	1,000.00	67.02	4,106.67	4,081.70
215	J508	80.58	1,000.00	60.48	4,076.57	1,961.95
216	J518	53.95	1,000.00	48.33	4,111.55	3,587.73
217	J52	105.59	1,000.00	89.65	4,084.89	2,832.37
218	J520	132.49	1,000.00	118.60	4,087.72	4,052.10
219	J522	114.23	1,000.00	70.21	4,018.03	1,576.60
220	J524	103.31	1,000.00	92.36	4,096.14	3,840.05
221	J526	87.48	1,000.00	66.45	4,071.37	2,088.38
222	J528	84.02	1,000.00	58.75	4,061.58	1,778.48
223	J530	62.35	1,000.00	33.03	4,052.22	1,257.57
224	J532	83.66	1,000.00	75.50	4,104.23	4,060.67
225	J534	62.28	1,000.00	30.17	4,045.63	1,187.67
226	J54	106.02	1,000.00	89.50	4,083.57	2,767.15
227	J56	104.29	1,000.00	89.04	4,086.48	2,902.25
228	J58	102.04	1,000.00	91.70	4,097.64	4,000.65
229	J60	100.67	1,000.00	89.40	4,095.33	3,700.08
230	J62	97.62	3,500.00	27.64	3,959.78	3,732.89
231	J64	95.01	1,000.00	84.55	4,097.14	3,817.67
232	J66	102.43	1,000.00	91.14	4,095.33	3,721.59
233	J68	91.98	1,000.00	82.16	4,098.62	3,988.99
234	J70	104.96	1,000.00	91.67	4,090.56	3,304.79

**Print Report**

	ID	Static Pressure (psi)	Fire-Flow Demand (gpm)	Residual Pressure (psi)	Residual Head (ft)	Available Flow @Hydrant (gpm)
235	J72	106.69	1,000.00	93.28	4,090.28	3,321.55
236	J74	99.31	1,000.00	86.67	4,092.02	3,312.03
237	J76	91.95	1,000.00	81.69	4,097.53	3,814.31
238	J78	92.84	1,000.00	82.88	4,098.28	3,958.12
239	J80	91.04	3,500.00	30.26	3,980.83	3,864.95
240	J82	90.61	1,000.00	80.58	4,097.96	3,881.86
241	J84	90.13	1,000.00	79.67	4,096.86	3,738.10
242	J86	84.50	1,000.00	71.95	4,092.06	2,970.95
243	J88	103.55	1,000.00	86.37	4,081.33	2,685.12
244	J90	91.42	1,000.00	80.34	4,095.41	3,557.26
245	J92	87.28	1,000.00	77.51	4,098.87	3,802.79
246	J94	87.32	1,000.00	77.60	4,099.10	3,818.63
247	J96	79.15	1,000.00	69.71	4,099.88	3,615.25
248	J98	78.19	1,000.00	67.72	4,097.28	3,224.94



# APPENDIX D

## ENGINEER'S OPINIONS OF PROBABLE COST

# Engineer's Opinion of Probable Cost

## SUNRISE ENGINEERING, INC.

11 North 300 West, Washington, Utah 84780

Tel: (435) 652-8450 Fax: (435) 652-8416

### Engineer's Opinion of Probable Cost

#### Install New 1.0 Million Gallon Tank, In Town Replacements

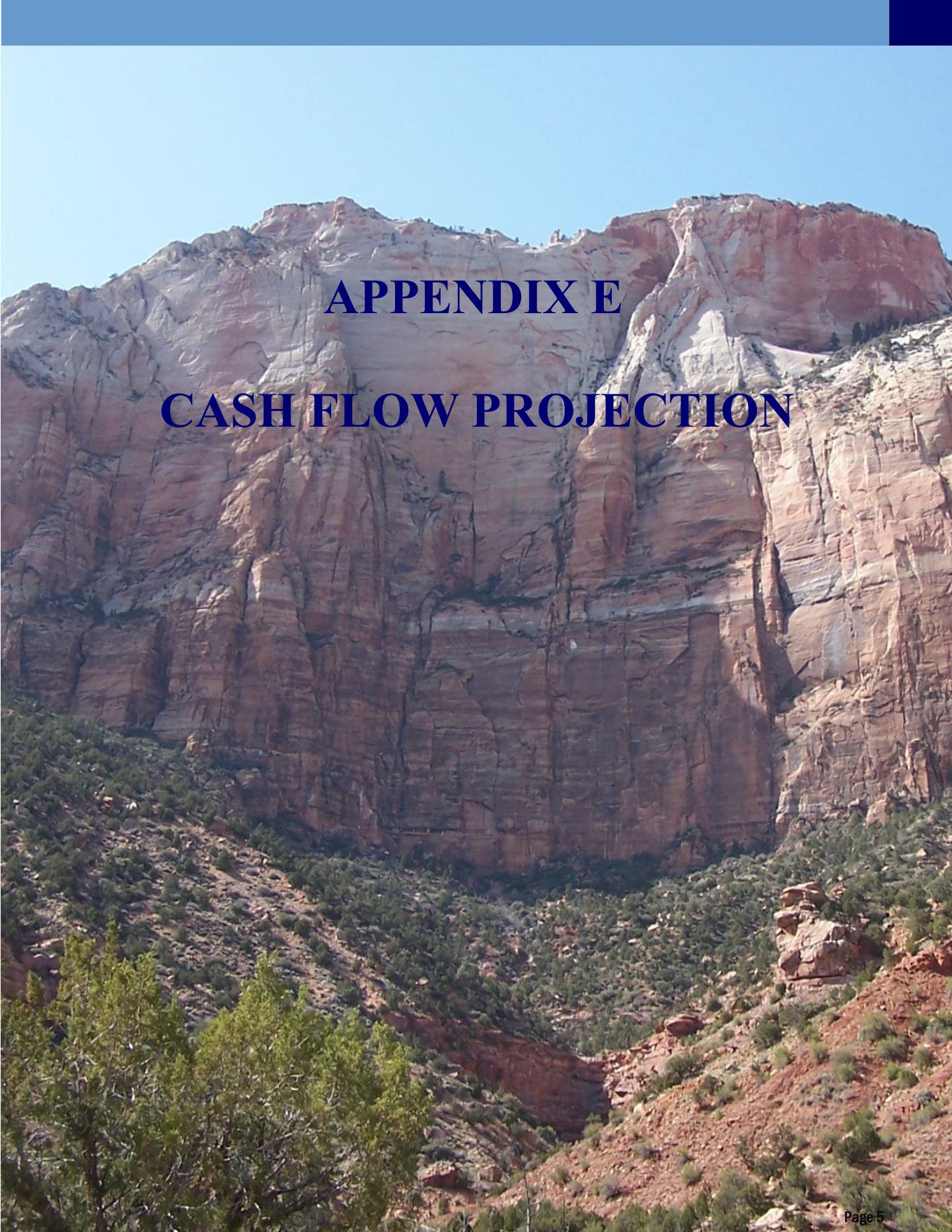
12-May-08

Town of Springdale

ALA

NO.	DESCRIPTION	Estimated Quantity	Units	Unit Price	TOTAL COST
1	Mobilization	1	LS	\$ 96,000.00	\$ 96,000.00
2	Earthwork 1.0 Million Gallon Tank	1	LS	\$ 85,000.00	\$ 85,000.00
3	Construct 1.0 Million Gallon Tank	1	LS	\$ 800,000.00	\$ 800,000.00
4	Tank Appurtenances	1	LS	\$ 75,000.00	\$ 75,000.00
5	Chainlink Fence & Gate	800	LN.FT.	\$ 22.00	\$ 17,600.00
6	Metering Station	1	LS	\$ 25,000.00	\$ 25,000.00
7	8" PVC Line & Fittings, Replace transite pipe, Foot Hill Lane	1,750	Ln. Ft.	\$ 21.00	\$ 36,750.00
8	8" Gate Valve Assembly	18	Each	\$ 1,200.00	\$ 21,600.00
9	Fire Hydrant Reconnection	16	Each	\$ 2,000.00	\$ 32,000.00
10	Fire Hydrant Assembly	4	EA.	\$ 3,500.00	\$ 14,000.00
11	10" PVC Line & Fittings, Paradise and Zion Park Boulevard	7,750	Ln. Ft.	\$ 27.00	\$ 209,250.00
12	10" Gate Valve Assembly	19	Each	\$ 1,800.00	\$ 34,200.00
13	12" PVC Line & Fittings, New Tank to Highway	5,250	LN.FT.	\$ 33.00	\$ 173,250.00
14	12" Gate Valve Assembly	5	EA.	\$ 2,200.00	\$ 11,000.00
15	Untreated Base Course (6" Depth in Trench)	91,163	SQ. FT.	\$ 0.85	\$ 77,488.55
16	Bituminous Surface Course	91,163	SQ. FT.	\$ 2.15	\$ 196,000.45
17	Reconnection of Meters (saddle, meter setter & service lateral pipe)	37	Each	\$ 900.00	\$ 33,300.00
18	Retrofitting the existing booster pumps to pump to the new tank	1	LS	\$ 93,000.00	\$ 93,000.00
19					
20	<b>Sub-Total</b>				\$ 2,030,400
21	<b>Contingency</b>	14%			\$ 284,300
22	<b>Total Construction</b>				\$ 2,314,700
23					
24	<b>INCIDENTALS</b>				
25	Funding & Administrative Services	1%	L.S.	\$ 23,100	
26	Legal and Fiscal		Est.	\$ 69,800	
27	Engineering Design	5.3%	L.S.	\$ 156,000	
28	Engineering Construction Services	6.2%	Hourly	\$ 185,200	
29	SWPPP (Storm Water Pollution Protection Plan)		Est.	\$ -	
30	Environmental/Archeology		Est.	\$ 20,000	
31	Geotechnical Engineering		Est.	\$ 30,000	
32	Electrical Engineering		Est.	\$ 15,000	
33	Land & R/W Acquisition/Negotiation		Est.	\$ 20,000	
34	Water Rights Research and POD Applications		Est.	\$ 10,000	
35	Survey & GIS Mapping		Est.	\$ 20,000	
36	Radio Read Meters/Equipment/Software - Materials, no Install		Est.	\$ 40,000	
37	SCADA Improvements		Est.	\$ 45,000	
38	Miscellaneous Engineering Services		Est.	\$ 20,000	
39					
40					
41	<b>TOTAL PROJECT COST</b>				\$ 2,968,800

*In providing opinions of probable construction cost, the Client understands that the Engineer has no control over costs or the price of labor, equipment or materials, or over the Contractor's method of pricing, and that the opinion of probable construction cost provided herein is made on the basis of the Engineer's qualifications and experience. The Engineer makes no warranty, expressed or implied, as to the accuracy of such opinions compared to bid or actual costs.*



# APPENDIX E

## CASH FLOW PROJECTION

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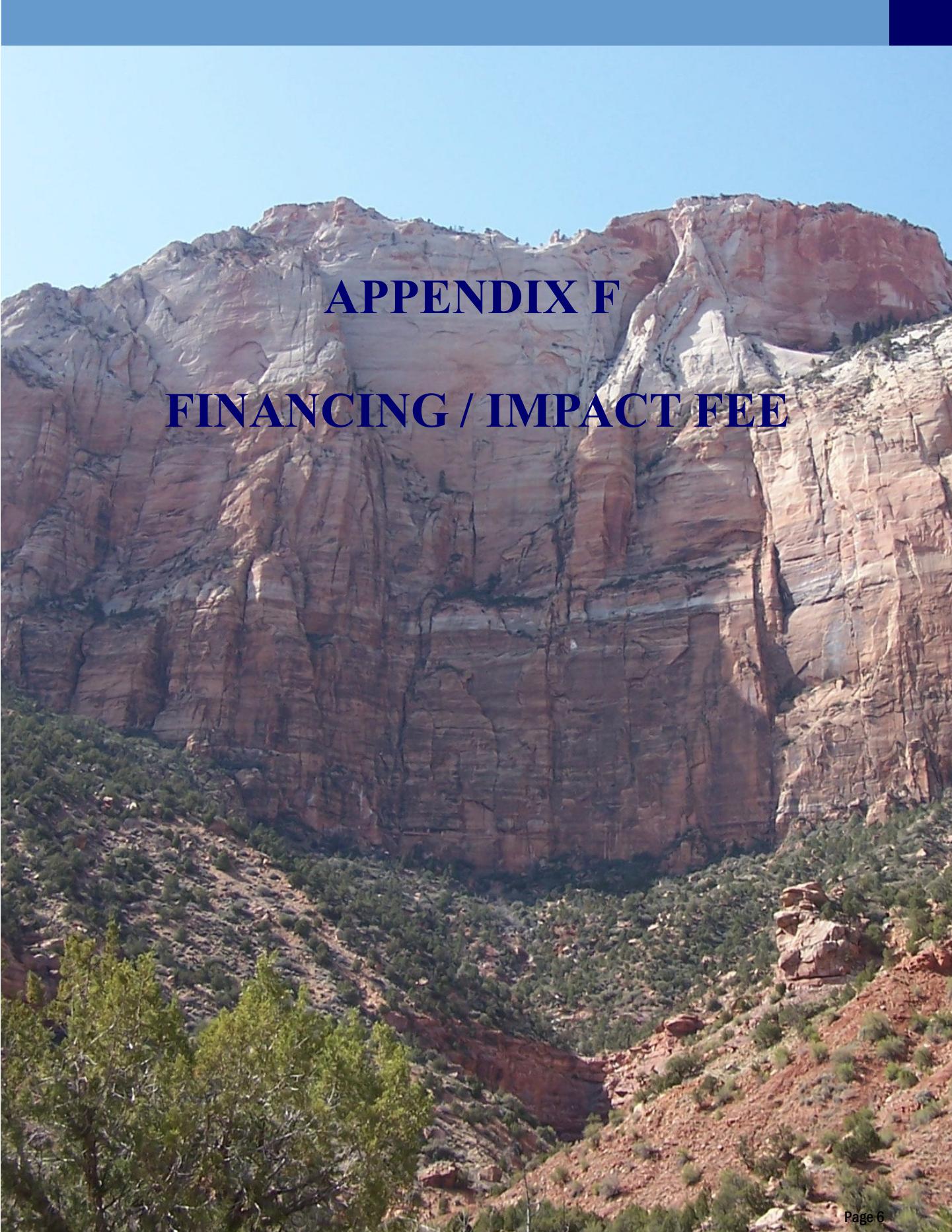
## Cash Flow Analysis

## Cash Flow Analysis

## Cash Flow Analysis

## Cash Flow Analysis

	Fiscal Year Beginning July 1 Ending June 30	2027	2028	2029
1	Average Rate ERU	\$74.41	\$76.83	
2	Connection Fee	\$540	\$540	
3	Impact fee	\$ 5,842	\$ 5,842	
4	<b>System Users:</b>			
5	Total Existing ERU's	1,287	1,316	
6	New ERU's:	38	29	
7				
8	<b>REVENUES:</b>			
9	User Fees (Water Sales)	1,166,125	1,226,611	
10	Impact and Connection Fees	121,256	92,537	
11	Late Fees & Penalties	0	0	
12	Miscellaneous	13,929	14,416	
13	<b>Garbage collection revenue</b>			
14	Interest Income/Bond Discount	9,949	10,297	
15	<b>TOTAL REVENUE:</b>	<b>\$1,311,258</b>	<b>\$1,343,862</b>	
16				
17	<b>EXPENSES:</b> (Inc. O&M & Debt Serv.)			
18	Salaries, wages and benefits	310,863	326,562	
19	Office expenses and travel	16,788	17,636	
20	Repairs and maintenance	100,787	105,877	
21	Utilities	36,643	38,494	
	Legal and professional fees	32,541	34,184	
	Garbage collection			
	Contract services	6,227	6,542	
	Insurance	27,383	28,765	
22	Miscellaneous	38,847	40,808	
23	Depreciation	0	0	
24	<b>Sub-Total Operation &amp; Maintainance</b>	<b>\$570,079</b>	<b>\$598,868</b>	
25				
26	<b>EXISTING DEBT SERVICE (810-820)</b>			
27	No Interest State Loan	0	0	
28	No Interest State Loan (Payment/10)	0	0	
29	Water Revenue Bond 1995A	0	0	
30	Water Revenue Bond 1995 Reserve (Payment/6)	0	0	
	1995A DSRF			
	1995A DSRF Reserve (Payment/10)			
	Water Revenue Bond 2004 (tank project)			
	Water Revenue Bond 2004 Reserve (Payment/6)			
31	Owing to the General Fund			
32	<b>Sub-Total Existing Debt Service</b>	<b>\$0</b>	<b>\$0</b>	
33				
34	<b>NEW DEBT SERVICE (810-820)</b>			
35	2008 Loan	77,496	77,496	
36	Loan Reserve (Payment/10)	0	0	
37				
38	2016 Loan	70,361	70,361	
39	Loan Reserve (Payment/10)	0	1	
40				
41	<b>Sub-Total New Debt Service</b>	<b>\$147,857</b>	<b>\$147,858</b>	
42	<b>Total Debt Service</b>	<b>\$147,857</b>	<b>\$147,858</b>	
43				
44	Renewal and Replacement Fund (590)	157,014	164,944	
45	<b>TOTAL EXPENSES:</b>	<b>\$874,951</b>	<b>\$911,670</b>	
46				
47	<b>Net Cashflow</b>	<b>\$436,308</b>	<b>\$432,192</b>	
48				
49	<b>CASH ON HAND</b>			
50	*Fund Balance	2,242,893	2,675,085	
51	Renewal and Replacement Account Balance:	2,065,870	2,230,814	
52	New Bond Reserves	105,000	105,000	
53	<b>Total</b>	<b>\$4,413,764</b>	<b>\$5,010,899</b>	
54	<i>*Fund Balance is obtained by adding the previous year's balance to the net cash flow, minus any self funded portion of future projects.</i>			
55				
56				
57	<b>Total Project Amount</b>	<b>0</b>	<b>0</b>	
58	<b>FINANCING PLAN FOR PROJECT PHASES:</b>			



# APPENDIX F

## FINANCING / IMPACT FEE

Page 6

## Proposed Financing Plan

<b>TOWN OF SPRINGDALE</b>			
<b>FY 2009 PROPOSED FINANCING PLAN</b>			
<b>TOTAL PROJECT COST</b>			<b>\$ 2,968,600</b>
<b>FY 2009 EXPENSES</b>			
<b>Proposed Funding:</b>	<b>Rate</b>	<b>Term in Yrs.</b>	<b>Principal</b>
Self Participation			200,000
DWB Grant			768,600
DWB Loan	1.00%	30	2,000,000
<b>TOTAL PROJECT FUNDING:</b>			<b>\$2,968,600</b>
<b>EXPENSES: (First Year of New Debt Serv. Pmt.)</b>			
Salaries, wages and benefits			\$121,911
Office expenses and travel			\$6,584
Repairs and maintenance			\$39,526
Utilities			\$14,370
Legal and professional fees			\$12,761
Garbage collection			\$0
Contract services			\$2,442
Insurance			\$10,739
Miscellaneous			\$15,235
Depreciation			\$0
	<b>Subtotal Expenses:</b>	<b>\$223,568</b>	
<b>EXISTING DEBT SERVICE</b>			
No Interest State Loan	0%		\$17,080
Loan Reserve (Payment /10)			\$1,708
Water Revenue Bond 1995A	0%	20	\$48,000
Bond Reserve (Payment/6)			\$8,000
Water Revenue Bond 2004 Tank Project	1%	20	\$21,260
Bond Reserve (Payment/6)			\$10,000
	<b>Subtotal Existing Annual Debt Service:</b>	<b>\$106,048</b>	
<b>NEW DEBT SERVICE</b>			
New Loan(s)	100.0%	0	\$77,496
Loan Reserve (Payment/10)			\$7,700
	<b>Subtotal New Annual Debt Service:</b>	<b>\$85,196</b>	
Renewal and Replacement Fund			\$66,000
	<b>GRAND TOTAL EXPENSES:</b>	<b>\$480,812</b>	
<b>ANNUAL INCOME</b>			
Transient Room Tax Revenues			\$60,000
Projected Yearly Impact Fees Received			\$70,201
Total Number Of <u>ERU's</u>			721
Average Monthly Water User Rate/ERU			\$40.52
	<b>TOTAL ANNUAL INCOME:</b>	<b>\$480,812</b>	
Average Monthly Irrigation Water User Rate/ERU			\$4.88
Total Average Monthly Water Cost/ERU			\$45.40

## Impact Fee Analysis

<b>IMPACT FEE ANALYSIS FY2008</b>		
<b>CULINARY WATER MASTER PLAN</b>		
Feb-08		
<b>EXISTING DEBT SERVICE</b>	<b>% Eligible</b>	<b>Eligible</b>
No Interest State Loan	100%	\$ 102,480
Water Revenue Bond 1995A	100%	\$ 306,000
Water Revenue Bond 2004 Tank Project	100%	\$ 1,014,890
Existing Impact Fee Eligible Cost:		<u>\$ 1,423,370</u>
<b>PROPOSED IMPROVEMENT PROJECTS</b>		
Total Estimated 2008 Project Cost		\$ 2,968,800
Total Estimated 2008 Project Cost Paid in Grant Funds		\$ (768,800)
% Of New Project Cost Due to New Growth	85.2%	\$ 1,874,980
Interest From New Debt Service	\$324,887	<u>\$ 276,889</u>
Impact Fee Eligible Cost		\$ 2,151,869
No. of ERUs (2008 Historic)		699
Future ERU's (2028 Historic)		<u>1,289</u>
No. of New ERU's Due to Growth		590
Impact Fee Eligible Cost for Improvement Projects		\$ 3,575,239
Impact Fee Amount for Improvement Projects = Total Eligible Cost / New ERU's		\$ <u>6,060</u> /Conn.
Additional Amount per connection for Future Water Rights		\$ <u>-</u> /Conn.
Proposed Impact Fee for Town of Springdale Water Users (FY2008) =		<b><u>\$ 6,060</u> /Conn.</b>
<b>Size</b>	<b>Sectional</b>	<b>Increase</b>
3/4"	0.44	0%
1"	0.79	80%
1 1/2"	1.77	302%
2"	3.14	614%
3"	7.07	1507%
4"	12.57	2757%
6"	28.27	6325%
		<b>Impact Fee</b>
3/4"	0.44	\$ 6,060
1"	0.79	\$ 10,880
1 1/2"	1.77	\$ 24,377
2"	3.14	\$ 43,244
3"	7.07	\$ 97,369
4"	12.57	\$ 173,115
6"	28.27	\$ 389,338